

GATE-2014 Exam Solutions Page MADE E India's Best Institute for IES. GATE & PSUs **Civil Engineering** (Morning Session) 2  $= \lim_{h \to 0} 1 + \left(\frac{\sin \frac{1}{h}}{\frac{1}{h}}\right) = 1$ End of Solution **Q.4** A conventional flow duration curve is a plot between (a) Flow and % time flow is exceeded (b) Duration of flooding and ground level elevation (c) Duration of water supply in a city and proportion of area recurring supply exceeding this duration. (d) Flow rate and duration of time taken to empty of a reservoir at that flow rate. Ans. (a) End of Solution Q.5 A steel section is subjected to a combination of shear and bending action. The applied shear force is V and shear capacity of the section is V<sub>s</sub>. For such a section, high shear force (as per IS 800-2007) is defined as (a)  $V > 0.6 V_s$ (b)  $V > 0.7 V_s$ (d) V > 0(c)  $V > 0.8 V_s$ Ans. (a) Clause 9.2.1 IS 800:2007. End of Solution Q.6 In reservoir with an uncontrolled spillway the peak of the plotted outflow hydrograph (a) Lies outside the plotted inflow hydrograph. (b) Lies on the recession limb of the plotted inflow hydrograph. (c) Lies on the peak of the inflow hydrograph. (d) is higher than peak of the plotted inflow hydrograph. Ans. (b) Inflow hydrograph Recession limb Outflow hydrograph





**Q.11** The monthly rainfall chart based on 50 years of rainfall in Agra is shown in the following figure which of the following are true ? (K percentile is the value such that K % of data fall below that value)



- (i) On average it rains more in July than in Dec.
- (ii) Every year, the amount of rainfall in August is more than that in January.

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|--------|--------------------------------------------------------------------------------------------------------------------------------|---------------------------------------------------------------------------------|---------------|
|        | <ul><li>(iii) July rainfall can be estimated v</li><li>(iv) In Aug, there is at least 500 r</li><li>(a) (i) and (ii)</li></ul> | with better confidence than Feb. rainfa<br>nm of rainfall.<br>(b) (i) and (iii) | all.          |
|        | (c) (ii) and (iii)                                                                                                             | (d) (iii) and (iv)                                                              |               |
| Ans. ( | b)                                                                                                                             |                                                                                 |               |
|        |                                                                                                                                | • • End of Soluti                                                               | ion           |
| Q.12   | The potable water is prepared from foil treatment square.                                                                      | m turbid surface water by adopting t                                            | the           |
|        | (a) Turbid surface water $\rightarrow$ Coagula<br>Filtration $\rightarrow$ Disinfection $\rightarrow$ St                       | ation $\rightarrow$ Flocculation $\rightarrow$ Sedimentation orage and supply   | $\rightarrow$ |
|        | (b) Turbid surface water $\rightarrow$ Disinfer<br>Filteration $\rightarrow$ Coagulation $\rightarrow$ S                       | ction $\rightarrow$ Flocculation $\rightarrow$ Sedimentation torage and supply  | $\rightarrow$ |
|        | (a) Turbid surface water $\rightarrow$ Filtera<br>Flocculation $\rightarrow$ Coagulation                                       | ation $\rightarrow$ Sedimentation $\rightarrow$ Disinfection                    | $\rightarrow$ |
|        | (a) Turbid surface water $\rightarrow$ Sedime<br>Disinfection $\rightarrow$ Filteration                                        | ntation $\rightarrow$ Flocculation $\rightarrow$ Coagulation                    | $\rightarrow$ |
| 0.19   | The minimum value of 15 minutes                                                                                                | ••• End of Solution                                                             | ion           |
| Q.13   | is                                                                                                                             | peak nour factor on a section of a re                                           | Jau           |
|        | <ul><li>(a) 0.1</li><li>(c) 0.25</li></ul>                                                                                     | (b) 0.2<br>(d) 0.33                                                             |               |
| Ans. ( | c)                                                                                                                             |                                                                                 |               |
|        | 15 min. peak hr factor is used fo                                                                                              | r traffic intersection design                                                   |               |
|        | $PHF = \frac{(V/4)}{V_{15}}$                                                                                                   |                                                                                 |               |
|        | V = Peak hourly v                                                                                                              | volume $\left( in \frac{veh.}{hr.} \right)$                                     |               |
|        | V <sub>15</sub> = Maximum 15<br>(veh.)                                                                                         | ominimum volume within the peak                                                 | hr.           |
|        | Maximum value is 1.0 and minimu                                                                                                | um value is 0.25                                                                |               |
|        | Normal range is $0.7 - 0.98 = 0.25$                                                                                            |                                                                                 |               |
|        |                                                                                                                                |                                                                                 |               |
|        |                                                                                                                                | • • End of Soluti                                                               | ion           |





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|--------------|----------------------------------------------------------|-----------------------------------------------------------------|-----|
| Q.19         | Match the following:                                     |                                                                 |     |
| •            | Group I                                                  | Group II                                                        |     |
|              | P. Alidade                                               | 1. Chain Survey                                                 |     |
|              | Q. Arrow                                                 | 2. Levelling                                                    |     |
|              | R. Bubble tube                                           | 3. Plant table surveying                                        |     |
|              | S. Stedia hair                                           | 4. Theodolite                                                   |     |
|              | (a) $P - 3$ , $Q - 2$ , $R - 1$ , $S - 4$                |                                                                 |     |
|              | (b) $P - 2$ , $Q - 4$ , $R - 3$ , $S - 1$                |                                                                 |     |
|              | (c) $P - 1$ , $Q - 2$ , $R - 4$ , $S - 3$                |                                                                 |     |
|              | (d) $P - 3$ , $Q - 1$ , $R - 2$ , $S - 4$                |                                                                 |     |
| Ans.         | (d)                                                      |                                                                 |     |
|              | P - 3, Q - 1, R - 2, S - 4                               |                                                                 |     |
|              |                                                          |                                                                 |     |
| 0.90         | The sum of eigen value metric                            |                                                                 | on  |
| Q.20         | The sum of eigen value matrix                            |                                                                 |     |
|              | 215                                                      | 650 795]                                                        |     |
|              | When [M] = 655                                           | 150 835                                                         |     |
|              | 485                                                      | 355 550                                                         |     |
|              | (-) 015                                                  | (L) 1955                                                        |     |
|              | (a) $913$<br>(c) $1640$                                  | (d) 2180                                                        |     |
|              |                                                          | (4) 2100                                                        |     |
| Ans.         | (a)                                                      |                                                                 |     |
|              | Sum of eigen value                                       | es = trace of matrix                                            |     |
|              |                                                          | = 215 + 150 + 550 = 915                                         |     |
|              |                                                          | • • End of Soluti                                               | on  |
| <b>Q</b> .21 | The probability density function                         | of evaporation E on any day during a ve                         | ear |
|              | in watershed is given by                                 |                                                                 |     |
|              | $\begin{bmatrix} 1 \\ 0 \end{bmatrix}$                   | $\leq E \leq mm/day$                                            |     |
|              | $f(E) = \begin{cases} 5 \\ 0 \end{cases}$                | Othomico                                                        |     |
|              | [0                                                       | Other wise                                                      |     |
|              | The probability that E lies in betw<br>is (in decimal)   | veen 2 and 4 mm/day in a day in watersh                         | led |
| Sol.         |                                                          |                                                                 |     |
|              | ٢.                                                       |                                                                 |     |
|              | $\left \frac{1}{5}\right  0 \leq$                        | $E \le mm/day$                                                  |     |
|              | $f(E) = \begin{cases} 0 \\ 0 \end{cases}$                | Otherwise                                                       |     |
|              | Į v                                                      |                                                                 |     |
|              |                                                          |                                                                 |     |
|              |                                                          |                                                                 |     |

P (2 < E < 4) = 
$$\int_{2}^{4} f(E) dE = \int_{2}^{4} \frac{1}{5} dE = \frac{1}{5} [E]_{2}^{4}$$
  
=  $\frac{1}{5} (4-2) = \frac{2}{5} = 0.4$ 

**Q.22** A box of weight 100 kN shown in the figure to be lifted without swinging. If all the forces are coplanar, the magnitude and direction ( $\theta$ ) of force F w.r.t. x axis is \_\_\_\_\_.



(a) F = 56.389 kN and  $\theta$  = 28.28°

(b) F = -56.389 kN and  $\theta = -28.28^{\circ}$ 

(c) F = 9.055 kN and  $\theta$  = 1.1414°

(d) F = -9.055 kN and  $\theta$  = -1.1414°



**Q.23** The amount of  $CO_2$  generated in kg while completely oxidizing one kg of  $CH_4$  is \_\_\_\_\_.

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Page 9 Sol.

$$CH_4 + 2O_2 \rightarrow CO_2 + 2H_{2C}$$

$$_{16g}$$

 $\Rightarrow~16~{\rm g}$  of  ${\rm CH}_4$  when completely oxidized leads to 44 g of  ${\rm CO}_2$ 

- $\Rightarrow$  1 kg of CH<sub>4</sub> when completely oxidized leads to  $\frac{44}{16} \times 1 = 2.75 \text{ kg CO}_2$ 
  - End of Solution

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Q.24 While designing for a steel column of Fe250 grade the base plate resting on a concrete pedestal of M20 grade, the bearing strength of concrete (N/mm<sup>2</sup>) in LSM of design as per IS 456: 2000 is ......

Sol.

Permissible bearing stress =  $0.45 f_{ck}$ 

 $= 0.45 \times 20 = 9$  N/mm<sup>2</sup>

• • • End of Solution

**Q.25** Given 
$$J = \begin{bmatrix} 3 & 2 & 1 \\ 2 & 4 & 2 \\ 1 & 2 & 6 \end{bmatrix}$$
 and  $K = \begin{bmatrix} 1 \\ 2 \\ -1 \end{bmatrix}$  then product  $K^T$  JK is \_\_\_\_\_.

Sol.

$$J = \begin{bmatrix} 3 & 2 & 1 \\ 2 & 4 & 2 \\ 1 & 2 & 6 \end{bmatrix}, K = \begin{bmatrix} 1 \\ 2 \\ -1 \end{bmatrix}$$
$$K^{T} JK = \begin{bmatrix} 1 & 2 & -1 \end{bmatrix} \begin{bmatrix} 3 & 2 & 1 \\ 2 & 4 & 2 \\ 1 & 2 & 6 \end{bmatrix} \begin{bmatrix} 1 \\ 2 \\ -1 \end{bmatrix}$$
$$= \begin{bmatrix} 6 & 8 & -1 \end{bmatrix} \begin{bmatrix} 1 \\ 2 \\ -1 \end{bmatrix} = 6 + 16 + 1 = 23$$

End of Solution

### **Two Marks Questions**

**Q.26** Three rigid bucket are of identical height and base area. Further assume that each of these buckets have negligible mass and are full of water. The weight of water in these bucket are denoted by  $W_1$ ,  $W_2$ ,  $W_3$  respectively. Which of the following option are correct.



$$\frac{d^2y}{dx^2} + \frac{py}{EI} = 0$$

the mid-span deflection of a member shown in figure is



[a is the amplitude constant for y]

(a) 
$$y = \frac{1}{P} \left( 1 - a \cos \frac{2\pi x}{L} \right)$$
  
(b)  $y = \frac{1}{P} \left( 1 - a \sin \frac{2\pi x}{L} \right)$   
(c)  $y = \frac{a \sin n \pi x}{L}$   
(d)  $y' = \frac{a \cos n \pi x}{L}$ 



Ans. (c)

| $\frac{\mathrm{d}^2 \mathrm{y}}{\mathrm{d} \mathrm{x}^2} =$ | $= -\frac{P}{EI} \times y$                                              |
|-------------------------------------------------------------|-------------------------------------------------------------------------|
| =                                                           | = -m <sup>2</sup> y                                                     |
| :. Solution of above di                                     | ifferential equation is                                                 |
| y =                                                         | a sin mx + b cos mx                                                     |
| at x =                                                      | = 0, y = 0                                                              |
| $\Rightarrow$ b =                                           | = 0                                                                     |
| at x =                                                      | = L, y = 0                                                              |
| $\Rightarrow$ 0 =                                           | = sin mL                                                                |
| $\Rightarrow$ mL =                                          | = $n\pi$                                                                |
| ⇒ m =                                                       | $=\frac{n\pi}{L}$                                                       |
| ∴ y =                                                       | $= \operatorname{asin} \frac{\operatorname{n} \pi x}{\operatorname{L}}$ |
|                                                             | • • • End of Solution                                                   |

**Q.28** A rectangular beam of 230 mm width and effective depth = 450 mm, is reinforced with 4 bars of 12 mm diameter. The grade of concrete is M 20, grade of steel is Fe 500. Given that for M 20 grade of concrete, the ultimate shear strength  $\tau_{uc} = 0.36 \text{ N/mm}^2$  for steel percentage of = 0.25, and  $\tau_{uc} = 0.48 \text{ N/mm}^2$  for steel percentage = 0.5. For a factored shear force of 45 kN, the diameter (mm) of Fe 500 steel 2 legged stirrups to be used at spacing of 325 mm should be

(c) 12 (d) 16

Ans. (a)



$$\begin{split} \tau_{uc} &= 0.36 \ \text{N/mm}^2 \left[\text{For M20}\right] \ \text{for} \ \frac{A_{st}}{bd} \times 100 = 0.25 \\ \tau_{uc} &= 0.48 \ \text{N/mm}^2 \left[\text{For M20}\right] \ \text{for} \ \frac{A_{st}}{bd} \times 100 = 0.5 \\ \text{Factored SF} &= 45 \ \text{kN} = V_u \end{split}$$

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We have to calculate the dia of Fe 500 2-L egged stirrup to be used at a spacing of 325 mm c/c

$$\tau_v = \frac{V_u}{bd} = \frac{45 \times 1000}{230 \times 450} = 0.4348 \text{ N/mm}^2$$

% tensile steel = 
$$\frac{4 \times \frac{\pi}{4} (12)^2}{230 \times 450} \times 100 = 0.437\%$$

$$\tau_{c} = 0.36 + \frac{0.12}{0.25} \times (0.437 - 0.25) = 0.45 \,\text{N/mm}^{2}$$

Since  $\tau_v - \tau_c < 0$ 

- $\Rightarrow$  Min shear reinforcement is required
- $\Rightarrow$  Min shear reinforcement is given by

$$\frac{\mathrm{A_{sv}}}{\mathrm{bS_v}} = \frac{0.4}{0.87 \, \mathrm{f_y}}$$

$$A_{sv} = \frac{0.4 \times (S_v)(b)}{0.87 f_y}$$

Since we limit  $f_v$  to 415 N/mm<sup>2</sup> hence,

$$A_{sv} = 2 \times \frac{\pi}{4} (\phi)^2 = \frac{0.4 \times 325 \times 230}{0.87 \times 415} = 82.814 \text{ mm}^2$$
  
 $\phi = 7.26 \text{ mm}$   
adopt  $\phi = 8 \text{ mm}$ 

• • End of Solution

**Q.29** 16 MLD of water is flowing through a 2.5 km long pipe of diameter 45 cm. The chlorine at the rate of 32 kg/d is applied at the entry of this pipe so that disinfected water is obtained at the exit. These is a proposal to increase the flow through the pipe to 22 MLD from 16 mLD. Assume the dilution coefficient n = 1. The minimum amount of chlorine (in kg per day) to be applied to achieve the same degree of disinfection for the enhanced flow is

| (a) | 60.5 | (b) | 4.4   |
|-----|------|-----|-------|
| (c) | 38   | (d) | 23.27 |

Ans. (a)

In the disinfection process we have the relationship,

 $tC^n = K$ 

where t = time required to kill all organism



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c = concentration of disinfectant

- n = dilution coefficient
- k = constant

=

 $\Rightarrow$ 

$$t_1 C_1^n = t_2 C_2^n$$
  
in our case  $n = 1$   
$$t_1 C_1 = t_2 C_2$$

 $\mathbf{t}_1 = \frac{\mathbf{L}}{\mathbf{v}_1}$ 

L = length of pipe ;  $V_1$  = velocity of flow

$$t_1 = \frac{L}{Q_1 / A}$$
$$t_1 = \frac{LA}{Q_1}$$

 $C_1=\frac{W_1}{Q_1}, \ {\rm where} \ W_1={\rm weight} \ {\rm of} \ {\rm disinfectant} \ {\rm per} \ {\rm day} \ ; \ Q_1={\rm discharge} \ {\rm per} \ {\rm day}$ 

$$\Rightarrow \qquad \frac{LA}{Q_1} \times \frac{W_1}{Q_1} = \frac{LA}{Q_2} \times \frac{W_2}{Q_2}$$
$$\Rightarrow \qquad W_2 = \frac{Q_2^2}{Q_1^2} \times W_1 = \left(\frac{22}{16}\right)^2 \times 32 \text{ kg/day} = 60.5 \text{ kg/day}$$

• • End of Solution

**Q.30** A rectangular channel flow have bed slope of 0.0001 width = 3 m coefficient n = 0.015,  $Q = 1 \text{ m}^3$ /sec given that normal depth of flow ranges between 0.76 m and 0.8 m. The minimum width of throat (in m) that is possible at a given section while ensuring that the prevailing normal depth does not exceed along the reach upstream of the concentration is approximately, equal to (assume negligible loss)

| (a) | 0.64 | (b) | 0.84 |
|-----|------|-----|------|
| (c) | 1.04 | (d) | 1.24 |

Ans. (b)

$$n = 0.015$$
  
 $Q = 1 m^{3/s}$ 

Normal depth of flow between 0.76 m to 0.8 m.

If prevailing normal depth of flow is not exceeded, there must not be chocking of the section or there must be just chocking.

Thus the width of the section should be such that for the prevailing specific energy there should be critical flow at the contracted section

i.e. 
$$\frac{3}{2} \left(\frac{q^2}{g}\right)^{1/3} = E_C = E_{initial}$$

$$\Rightarrow \qquad \frac{3}{2} \left[ \frac{\left(\frac{\mathbf{Q}}{\mathbf{B}_{\min}}\right)^2}{\mathbf{g}} \right]^{1/3} = \mathbf{E}_{\text{initial}}$$

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Let is now calculate E<sub>initial</sub>

$$Q = \frac{1}{2} A R^{2/3} S_0^{1/2}$$

$$1 = \frac{1}{0.015} (3y) \left(\frac{3y}{3+2y}\right)^{2/3} (0.0001)^{1/2}$$

 $\Rightarrow$ 

 $\Rightarrow$ 

$$y = 0.78 m$$

$$= 0.78 + \frac{\left(\frac{1}{3}\right)^2}{2 \times 9.81 \times (0.78)^2} = 0.7893 \text{ m}$$

$$\Rightarrow \qquad \frac{3}{2} \left[ \frac{\left(\frac{Q}{B_{\min}}\right)^2}{g} \right]^{1/3} = 0.7893$$
$$\Rightarrow \qquad \frac{3}{2} \frac{\left(1\right)^{2/3}}{g} = 0.7893$$

$$\Rightarrow \quad \frac{3}{2} \frac{(1)^{2/3}}{g^{1/3} (B_{\min})^{2/3}} = 0.7893$$
$$B_{\min} = 0.836 \text{ m}$$

End of Solution

Q.31 A levelling is carried out to established the reduced level (RL) of point R with respect to the bench mark (BM) at P. The staff reading taken are given below

Ans.

Ans.

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|      |                                                                 |                                                          | Staf                                                                              | f statio                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                        | n                                                | BS                                        | IS                                                    | FS                                                                   | RL                                              |                              |                     |                   |             |
|------|-----------------------------------------------------------------|----------------------------------------------------------|-----------------------------------------------------------------------------------|---------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------|-------------------------------------------|-------------------------------------------------------|----------------------------------------------------------------------|-------------------------------------------------|------------------------------|---------------------|-------------------|-------------|
|      |                                                                 |                                                          |                                                                                   | Р                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | 1                                                | .655                                      | -                                                     | -                                                                    | -                                               |                              |                     |                   |             |
|      |                                                                 |                                                          |                                                                                   | Q                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | -                                                | -0.95                                     | _                                                     | -1.5                                                                 | -                                               |                              |                     |                   |             |
|      |                                                                 |                                                          |                                                                                   | R                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                  | _                                         | -                                                     | 0.75                                                                 | -                                               |                              |                     |                   |             |
|      | If RL<br>(a) 10<br>(c) 10                                       | of P is +<br>3.355<br>1.455                              | - 100                                                                             | m, the                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          | en F                                             | RL (m                                     | n) of<br>(<br>(                                       | R is<br>b) 10<br>d) 10                                               | 3.155<br>0.355                                  |                              |                     |                   |             |
| Ans. | (c)                                                             |                                                          |                                                                                   |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 |                                                  |                                           |                                                       |                                                                      |                                                 |                              |                     |                   |             |
|      |                                                                 |                                                          |                                                                                   |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 |                                                  | HI =                                      | RL                                                    | , + B                                                                | S                                               |                              |                     |                   |             |
|      | and                                                             |                                                          |                                                                                   |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 |                                                  | RL =                                      | HI                                                    | – FS                                                                 | 5                                               |                              |                     |                   |             |
|      |                                                                 | Staff sta                                                | tion                                                                              | BS                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              | IS                                               | FS                                        | RI                                                    |                                                                      | HI                                              | RL                           |                     |                   |             |
|      |                                                                 | Р                                                        |                                                                                   | 1.655                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                           | I                                                | _                                         | -                                                     | 101                                                                  | .655                                            | 100                          |                     |                   |             |
|      |                                                                 | Q                                                        |                                                                                   | -0.95                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                           | -                                                | -1.5                                      | -                                                     | 102                                                                  | 2.205                                           | 103.155                      |                     |                   |             |
|      |                                                                 | R                                                        |                                                                                   | -                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | -                                                | 0.75                                      | -                                                     |                                                                      | _                                               | 101.455                      | l                   |                   |             |
|      | .:.                                                             |                                                          |                                                                                   | R                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | L of                                             | f R =                                     | 10                                                    | 1.455                                                                | m                                               |                              |                     |                   |             |
| Q.32 | A give<br>is com<br>the m<br>(a) 56<br>(c) 62<br>(d)<br>Relativ | en cohens<br>pacted to<br>ass densi<br>.43<br>.87<br>.87 | ionles<br>a m<br>ty of<br>$e_m$<br>$e_n$<br>$\rho_{fi}$<br>(<br>y, I <sub>D</sub> | ss soil $ass den$<br>ass den<br>water<br>water<br>ass denwater $ass denass denassdenass denass denass denass denass denass denass den$ | has<br>asity<br>as<br>85<br>5<br>800<br>000<br>7 | e <sub>max</sub><br>of 18<br>1000<br>kg/m | = 0.<br>800<br>kg/n<br>(<br>(<br>(<br><sup>3</sup> at | 85, e <sub>r</sub><br>kg/m <sup>3</sup> an<br>b) 60<br>d) 65<br>wate | nin =<br>at w<br>d G <sub>S</sub><br>.25<br>.41 | 0.5. In the ater cont = 2.7. | e field<br>ent of 8 | , the s<br>8%. Ta | ;oil<br>ıke |
|      |                                                                 |                                                          | 1 .                                                                               | $\rho = \frac{G}{2}$                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                            | $\frac{1}{1}$                                    | $\frac{1 + w}{+ e}$                       | $\frac{)}{2} = \frac{2}{2}$<br>× 1.0                  | $\frac{2.7 \times 10}{1}$                                            | 000 (1<br>. + e                                 | .08)                         |                     |                   |             |
|      | $\Rightarrow$                                                   |                                                          | 1 +                                                                               | e = -                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                           |                                                  | 1800                                      |                                                       | _                                                                    |                                                 |                              |                     |                   |             |
|      | $\Rightarrow$                                                   |                                                          |                                                                                   | e = 0.                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          | 62                                               |                                           |                                                       |                                                                      |                                                 |                              |                     |                   |             |
|      | $\Rightarrow$                                                   |                                                          |                                                                                   | $I_D = \frac{1}{e_1}$                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                           | e <sub>max</sub><br><sub>max</sub> -             | $\frac{x - e}{-e_{\min}}$                 | -×10                                                  | 00                                                                   |                                                 |                              |                     |                   |             |
| Corp | orate Office: 4                                                 | 14-A/ <u>1, Kalu S</u> a                                 | arai, <u>Ne</u>                                                                   | w D <u>elhi-16</u>                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              | 5 📋                                              | Email : r                                 | nadee                                                 | asyd <u>elh</u>                                                      | i@gmail                                         | l.com   <u>Visit:</u>        | www.mag             | leeasy.in         |             |
|      |                                                                 |                                                          |                                                                                   |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 |                                                  |                                           |                                                       |                                                                      |                                                 |                              |                     |                   |             |
|      |                                                                 |                                                          |                                                                                   |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 |                                                  |                                           |                                                       |                                                                      |                                                 |                              |                     |                   |             |







**Q.34** For a sample of water with the ionic composition shown below, the Carbonate and Non-carbonate hardness concentration (in mg/l as  $CaCO_3$ ) respectively are.



#### Ans. (c)

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Carbonate hardness =  $3.5\times10^{-3}$  g-eq [if NCH is present sodium alkalinity will be absent i.e. NaHCO\_3 absent]

$$= 3.5 \times 10^{-3} \times \frac{50 \,\mathrm{g}}{l} \mathrm{as} \,\mathrm{CaCO}_3$$

= 175 mg/l as  $CaCO_3$ 

Non carbonate hardness = total hardness-carbonate hardness Total hardness =  $5 \times 50 \text{ mg/}l$  as  $\text{CaCO}_3$  [total hardness is due to  $\text{Ca}^2$  + and  $\text{Mg}^{2+}$ ]

= 250 mg/l as CaCO3

$$\Rightarrow$$

# NCH = $250 - 175 = 75 \text{ mg/}l \text{ as } \text{CaCO}_3$











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 $u_{PR} \sin \theta = \frac{1}{2}$   $u_{PQ} + u_{PR} \cos \theta = 0$   $u_{PQ} = -u_{PR} \cos \theta = -\frac{1}{2\sin\theta} \cdot \cos\theta$   $u_{PQ} = -\frac{1}{2}\cot\theta = \frac{-1 \times 4/3}{2} = \frac{-2}{3}$   $\Delta_{R} = u_{PQ} \times \lambda_{PQ} = \frac{-2}{3}(-3) = 2 \text{ mm upwards}$ End of Solution

Q.40 The smallest angle of a triangle is equal to two third of the smallest angle of a quadrilateral. The ratio between the angle of the quadrilateral is 3:4:5:6. The largest angle of the triangle is twice its smallest angle, what is the sum, in degrees of the second largest angle of the triangle and the largest angle of the quadrilateral.

Sol.

 $\Rightarrow$ 



Largest angle of quadrilateral = 120° Smallest angle of quadrilateral = 60°

 $\Rightarrow \qquad \text{Smallest angle of triangle} = \frac{2}{3} \times (2 \times 20) = 40^{\circ}$ 

Largest angle of triangle =  $2 \times 40 = 80^{\circ}$ 

Third angle of triangle =  $60^{\circ}$ 

⇒ Sum of largest angle of quadrilateral and second largest angle of triangle =  $120^\circ$  +  $60^\circ$  =  $180^\circ$ 

• • • End of Solution

**Q.41** A straight 100 m long raw water gravity main is to carry water from intake to the jackwell of a water treatment plant. The required flow of water is  $0.25 \text{ m}^3$ /s. Allowable velocity through main is 0.75 m/s. Assume f = 0.01, g = 9.81. The minimum gradient (in cm/100 length) required to be given to this main so that water flow without any difficulty should be \_\_\_\_\_\_.

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Sol.

 $Q = 0.25 \text{ m}^{3}\text{/s}$ Allowable velocity = 0.75 m/s f = 0.01 g = 9.81  $\frac{\pi d^{2}}{4} = \frac{Q}{V} = \frac{0.25}{0.75} = \frac{1}{3} \text{ m}^{2}$   $\Rightarrow \qquad d = 0.6515 \text{ m}$   $\Rightarrow \qquad \frac{\text{flv}^{2}}{2\text{gd}} = \text{h}_{\text{f}} = \frac{0.01 \times 100 \times (0.75)^{2}}{2 \times 9.81 \times 0.6515} \text{ m}$  = 0.044 m = 4.4 cm  $\Rightarrow \text{ Minimum gradient} = \frac{\text{h}_{\text{f}}}{l} = \frac{4.4 \text{ cm}}{100 \text{ m}}$ Hence answer is 4.4.

• • • End of Solution

**Q.42** For a beam cross section W = 230 mm, effective depth = 500 mm, the number of reinforcement bars of 12 mm diameter required to satisfy minimum tension reinforcement requirement specified by IS-456-2000 (assume grade of steel is Fe500) is \_\_\_\_\_\_.

Sol.

$$\frac{A_{\text{st min}}}{bd} = \frac{0.85}{f_{y}}$$

$$A_{\text{st}} = \frac{0.85}{500} \times 230 \times 500 \text{ mm}^{2}$$

$$n = \frac{A_{\text{st}}}{\frac{\pi d^{2}}{4}} = \frac{0.85 \times 230}{\frac{\pi}{4}(12)^{2}}$$

$$= 1.729 = 2 \text{ bars}$$

• End of Solution

**Q.43** The perception - reaction time for a vehicle travelling at 90 km/h, given the coefficient of longitudinal friction of 0.35 and the stopping sight distance of 170 m (assume  $g = 9.81 \text{ m/s}^2$ ) is \_\_\_\_\_ seconds.

Sol.

 $\Rightarrow$ 

 $\Rightarrow$ 

SSD = 
$$0.278 \, \text{vt}_{r} + \frac{(0.278 \, \text{v})^2}{2 \text{gf}}$$

170 = 
$$0.278 \times 90 \times t_r + \frac{(0.278 \times 90)^2}{2 \times 9.81 \times 0.35}$$
  
 $t_r = 3.1510$  sec.

**Q.44** A traffic office impose on an average 5 number of penalties daily on traffic violators. Assume that the number of penalties on different day is independent and follows a Poisson distribution. The probability that there will be less than 4 penalties in a day is \_\_\_\_\_\_.

Sol.

Mean  $\lambda = 5$ 

P (x < 4) = p (x = 0) + p (x = 1) + p (x = 2) + p (x = 3) =  $\frac{e^{-5}s^{0}}{0!} + \frac{e^{-5}5^{1}}{1!} + \frac{e^{-5}5^{2}}{1!} + \frac{e^{-5}5^{3}}{3!}$ =  $e^{-5}\left[1 + 5 + \frac{25}{2} + \frac{125}{6}\right] = e^{-5}\left(\frac{118}{3}\right) = 0.265$ End of Solution

**Q.45** The speed-density (v-k) relationship on a single lane road with unidirectional flow is v = 70 - 0.7 K, where v is in km/hr and k is in veh/km. The capacity of the road (veh/hr) is

#### Ans. (a)

|                         | $Capacity = Velocity \times Density$                        |
|-------------------------|-------------------------------------------------------------|
| $\Rightarrow$           | $C = V \times K$                                            |
|                         | $= 70 \text{ K} - 0.7 \text{ K}^2$                          |
| Now,                    | $\frac{\mathrm{dC}}{\mathrm{dK}} = 70 - 1.4 \mathrm{K} = 0$ |
| $\Rightarrow$           | K = 50                                                      |
| $\Rightarrow$ Capacity, | $C = 70 \times 50 - 0.7(50)^2$                              |
|                         | = 1750 veh/hr                                               |
|                         |                                                             |

• • • End of Solution

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# GATE-2014 Exam Solutions Civil Engineering (Morning Session)

- Page 25
- **Q.46** A particle moves along a curve whose parametric equation are  $x = t^3 + 2t$ ,  $y = -3e^{\cdot 2t}$  and  $z = 2 \sin (5t)$ , where x, y and z show variation of the distance covered by the particles in (cm) with time (t) (in second). The magnitude of the acceleration of the particle (in cm/s<sup>2</sup>) at t = 0 is \_\_\_\_\_.

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Sol.

$$x = t^{3} + 2t$$

$$y = -3e^{-2t}$$

$$z = 2 \sin (5t)$$

$$\frac{dx}{dt} = 3t^{2} + 2$$

$$a_{x} = \frac{d^{2}x}{dt^{2}} = 6t$$

$$\frac{dy}{dt} = -3e^{-2t} \times (-2) = 6e^{-2t}$$

$$a_{y} = \frac{d^{2}y}{dt^{2}} = -12e^{-2t}$$

$$\Rightarrow \qquad \frac{dz}{dt} = -10 \times 5 \sin (5t) = -50 \sin 5t$$

$$a_{z} = \frac{d^{2}z}{dt^{2}} = -50\sin 5t$$

$$\vec{a} = a_{x}\hat{i} + a_{y}\hat{j} + a_{z}\hat{k}$$

$$\vec{a} \text{ at } t = 0 = 0\hat{i} - 12\hat{j} + 0\hat{k}$$

$$\vec{a} = -12\hat{j}$$

$$\Rightarrow Magnitude of acceleration at$$

$$t = 0 = 12 \text{ cm/s}^{2}$$

**Q.47** For a cantilever beam of a span 3m as shown a concentrated load of 20 kN applied to the free end causes a vertical displacement of 2mm at a section located at a distance of 1m from the fixed end (with no other load on the beam) the maximum vertical displacement in the same (in mm) is \_\_\_\_\_.





Sol.

Sum of the flow,  $y = y_1 + y_2 + y_3$  = 0.2 + 0.3 + 0.25 = 0.75Total lost time in a cycle L = 4 × 3 = 12 sec. Optimum cycle length,  $C_0 = \frac{1.5L+5}{1-y}$  $\frac{1.5 \times 12 + 5}{1-0.75} = 92$  sec.

**Q.49** Mathematical idealization of a crane has three bar with their vertices arranged as shown with load of 80 kN hanging vertically. The coordinate of the vertices are given in parenthesis. The force in member QR is \_\_\_\_\_.





GATE-2014 Exam Solutions MADE EASY India's Best Institute for IES. GATE & PSUs **Civil Engineering** (Morning Session)  $N_f = No.$  of flow channels = 3 Here,  $N_d$  = No. of equipotential drops = 10  $K = 3.8 \times 10^{-6} m/s$ Given, H = 6.3 mand  $Q = 3.8 \times 10^{-6} \times 6.3 \times \frac{3}{10}$ ...  $= 7.182 \times 10^{-6} \text{ m}^{3}/\text{s/m}$ =  $7.182 \times 10^{-6} \times 10^{6} \text{ cm}^{3}/\text{s/m}$ 

- • End of Solution
- **Q.51** The full data are given for laboratory sample  $\sigma'_0 = 175$  kPa,  $e_0 = 1.1 \sigma'_0 + \Delta \sigma'_0 = 300$  kPa, e = 0.9. If thickness of the clay specimen is 25 mm, the value of coefficient of volume compressibility is \_\_\_\_\_ × 10<sup>-4</sup> m<sup>2</sup>/kN.

 $Q = 7.182 \text{ cm}^3/\text{s/m}$ 

Sol.

$$m_{v} = \frac{a_{v}}{1 + e_{0}} = \frac{\frac{\Delta e}{\Delta \overline{o}}}{1 + e_{0}}$$
$$= \frac{(1.1 - 0.9)}{125 \times 2.1}$$
$$= 7.619 \times 10^{-4} \text{ m}^{2}/\text{kN}$$

End of Solution

**Q.52** The reinforced concrete section, the stress at extreme fibre in compression is 5.8 MPa. The depth of Neutral Axis in the section is 58 mm and grade of concrete is M25. Assuming Linear elastic behavior of the concrete, the effective curvature of the section (in per mm) is (a)  $2 \times 10^{-6}$  (b)  $3 \times 10^{-6}$ 

(c) 
$$4 \times 10^{-6}$$
 (d)  $5 \times 10^{-6}$ 



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Modulus of elasticity of concrete

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|               | $E = 5000\sqrt{25} = 25000 \text{ N/mm}^2$                                         |                 |
|---------------|------------------------------------------------------------------------------------|-----------------|
|               | $\frac{M}{I} = \frac{\sigma}{y} = \frac{E}{R}$                                     |                 |
| $\Rightarrow$ | $\frac{1}{R} = \frac{\sigma}{Ey} = \frac{5.8}{EX_u} = \frac{5.8}{58 \times 25000}$ |                 |
|               | = $4 \times 10^{-6}$ Per mm                                                        |                 |
| $\Rightarrow$ | Curvature = $4 \times 10^{-6}$ per mm                                              |                 |
|               |                                                                                    | End of Solution |

**Q.53** A venturimeter having diameter of 7.5 cm at the throat and 15 cm at the enlarged end is installed in a horizontal pipeline of 15 cm diameter. The pipe carries incompressible fluid at steady rate of 30 *l*/s. The difference of pressure head measured in terms of the moving fluid in between the enlarged and the throat of the vent is observed to be 2.45 m. Taking the  $g = 9.8 \text{ m/s}^2$ , the coefficient of discharge of venturimeter (correct upto 2 decimal) is

Sol.





$$= \frac{30 \times 30^{-3} \text{ m}^{3}/\text{s}}{\frac{\frac{\pi}{4} (0.15)^{2} \times (0.075)^{2}}{\sqrt{(0.15)^{2} - (0.075)^{2}}} \times \sqrt{2 \times 9.81 \times 2.45}}$$
  
C<sub>d</sub> = **0.95**

**Q.54** A traffic surveying conducted on a road yield an average daily traffic count of 5000 vehicle. The axle load distribution on the same road is given in the following table.

| Axle load (tones) | Frequency of traffic (f) |
|-------------------|--------------------------|
| 18                | 10                       |
| 14                | 20                       |
| 10                | 35                       |
| 8                 | 15                       |
| 6                 | 20                       |

The design period of the road is 15 years. The yearly Traffic growth rate is 7.5% the load safety factor (LSF) is 1.3. If the vehicle damage factor (VDF) is calculated from the above data, the design traffic (In million standard axle load MSA) is \_\_\_\_\_\_ .

Sol.

Calculation of vehicle damage factor

$$VDF = \frac{V_1 \left(\frac{W_1}{W_s}\right)^4 + V_2 \left(\frac{W_2}{W_3}\right)^y + V_3 \left(\frac{W_3}{W_s}\right)^4 + V_4 \left(\frac{W_4}{W_s}\right)^4 + V_5 \left(\frac{W_5}{W_s}\right)^4}{V_1 + V_2 + V_3 + V_4 + V_5}$$

where  $\rm W_s$  = standard axle load = 80 kN = 8.2 tonn

$$\Rightarrow \text{VDF} = \frac{10\left(\frac{18}{8.2}\right)^4 + 20\left(\frac{14}{8.2}\right)^4 + 35\left(\frac{10}{8.2}\right)^4 + 15\times\left(\frac{8}{8.2}\right)^4 + 20\times\left(\frac{6}{8.2}\right)^4}{10 + 20 + 35 + 15 + 20}$$
  
= 4.989  
$$\Rightarrow \text{ N} = \frac{365\times5000\left[(1.075)^{15} - 1\right]}{0.075} \times 4.989$$
  
= 237.806 MSA



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An incompressible fluid is flown at steady rate in horizontal pipe. From a Q.55 section the pipe divides into two horizontal parallel pipes of diameter  $(d_1 \text{ and } d_2)$  that run, for a distance of L each and then again join back to a pipe of the original size. For both the pipes, assume the head loss due to friction only and the Darcy-weisbach friction factor to be same. The velocity ratio between bigger and smaller branched pipe is \_

 $\Rightarrow$ 

Sol.



End of Solution

# Section - II (General Aptitude)

### **One Mark Questions**

Q.56 Rajan was not happy that Sajan decided to do the project on his own on observing his unhappiness, sajan explained to Rajan that he preferred to work independently.

> Which one of the statement below is logically valid and can be inferred from the above sentences?

- (a) Rajan has decided to work only in group.
- (b) Rajan and Sajan were formed into a group against their wishes.
- (c) Sajan decided to give into Rajan's request to work with him.
- (d) Rajan had believed that Sajan and he would be working together.

(d) Ans.

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|--------|--------------------------------------------------------------------------------------------------------------------------------------------------|---------------------------------------------------------------------------------------------------------------------------------------------------------------|------------------|
| Q.57   | A boundary has a fixed daily covariable cost of Rs. 8000 Q, where<br>is the cost of production in Rs. per                                        | st of Rs 50,000 whenever it operates an<br>e Q is the daily production in tonnes. Wh<br>r tonne for a daily production of 100 tonne                           | nd<br>nat<br>es. |
| Sol.   |                                                                                                                                                  |                                                                                                                                                               |                  |
|        | Total cost for 100 tonne                                                                                                                         |                                                                                                                                                               |                  |
|        | = 8000 ×                                                                                                                                         | 100 + 50000 = 850000                                                                                                                                          |                  |
|        | $Cost per tonne = \frac{850000}{100}$                                                                                                            | = Rs. <b>8500/tonne.</b>                                                                                                                                      |                  |
| Q.58   | Choose the most appropriate wor<br>the following sentences, one of his<br>(a) Vice<br>(c) Choices                                                | <ul> <li>end of Solution</li> <li>d from the option given below to complete s biggest was his ability to forgive (b) Virtues</li> <li>(d) Strength</li> </ul> | on<br>ete<br>ve. |
| Ans.   | (b)                                                                                                                                              |                                                                                                                                                               |                  |
| Q.59   | A student is required to demonst<br>subject, especially in the social<br>The word closes in meaning to<br>(a) Understanding<br>(c) Concentration | rate a high level of comprehension for t<br>sciences.<br>comprehension is<br>(b) Meaning<br>(d) Stability                                                     | he               |
| Ans.   | (a)                                                                                                                                              |                                                                                                                                                               | on               |
| Q.60   | Find the odd one in the followin<br>(a) ALRVX<br>(c) ITZDF                                                                                       | ng group ALRVX, EPVZB, ITZDF, OYEI<br>(b) EPVZB<br>(d) OYEIX                                                                                                  | IX               |
| Ans. ( | (d)                                                                                                                                              |                                                                                                                                                               |                  |
|        |                                                                                                                                                  | • • • End of Solution                                                                                                                                         | on               |
|        | Two Marks                                                                                                                                        | Questions                                                                                                                                                     |                  |
| Q.61   | One percent of the people of couthe people of country Y are taller<br>in country X as in country Y. T                                            | untry X are taller than 6 ft, 2 percent<br>than 6 ft. There are thrice as many peop                                                                           | of<br>ple        |
|        | people are taller than 6 ft?                                                                                                                     | aking both countries together. What 70                                                                                                                        | 01               |
|        | people are taller than 6 ft?<br>(a) 3                                                                                                            | (b) 2.5<br>(d) 1.25                                                                                                                                           | 01               |

