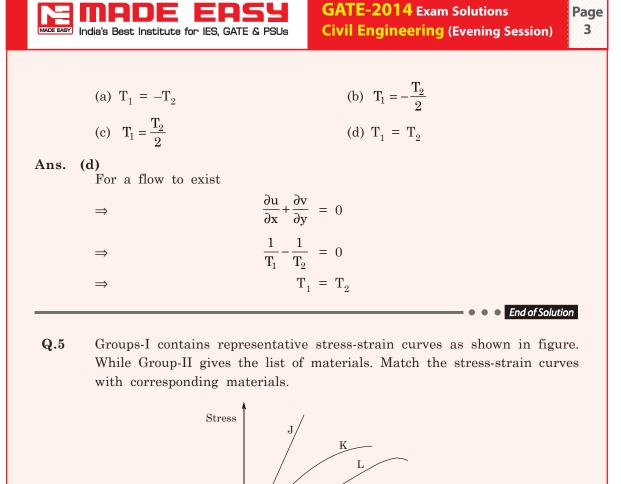
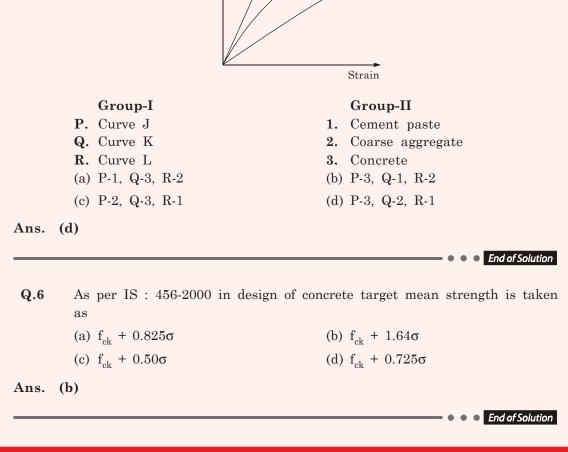


Section - I (Civil Engineering) **One Mark Questions** Q.1 If $G_s = 2.7\%$, n = 40%, w = 20% then the degree of saturation is _____. Sol. Se = wG $e = \frac{n}{1-n} = 0.67$ and $S = \frac{2.71 \times 20}{0.67} = 81.3\%$ \Rightarrow • • • End of Solution $\mathbf{Q.2}$ The determinant of matrix is _____. 0 1 2 3 Sol. $\Delta = \begin{vmatrix} 0 & 1 & 2 & 3 \\ 1 & 0 & 3 & 0 \\ 2 & 3 & 0 & 1 \\ 3 & 0 & 1 & 2 \end{vmatrix}$ $R_4 \rightarrow R_4 - R_2 - R_3$ | 0 1 2 3 $\Delta = \begin{vmatrix} 0 & 1 & 2 & 0 \\ 1 & 0 & 3 & 0 \\ 2 & 3 & 0 & 1 \\ 0 & -3 & -2 & 1 \end{vmatrix}$ $R_4 \rightarrow R_4 + 3R_1$ $\Delta = \begin{vmatrix} 0 & 1 & 2 & 3 \\ 1 & 0 & 3 & 0 \\ 2 & 3 & 0 & 1 \\ 0 & 0 & 4 & 10 \end{vmatrix}$ $\mathrm{R}_3 \rightarrow \mathrm{R}_3 - \, 3\mathrm{R}_1$ | 0 1 2 3 $\Delta = \begin{vmatrix} 1 & 0 & 3 & 0 \\ 2 & 0 & -6 & -8 \\ 0 & 0 & 4 & 10 \end{vmatrix}$

GATE-2014 Exam Solutions Page MADE ER India's Best Institute for IES. GATE & PSUs **Civil Engineering** (Evening Session) 2 Interchanging column 1 and column 2 and taking transpose $1 \ 0 \ 0 \ 0$ $\Delta = \begin{bmatrix} 0 & 1 & 2 & 0 \\ 2 & 3 & -6 & 4 \\ 3 & 0 & -8 & 10 \end{bmatrix}$ $= \begin{array}{ccc} -1 \times \begin{vmatrix} 1 & 2 & 0 \\ 3 & -6 & 4 \\ 0 & -8 & 10 \end{vmatrix}$ $= -1 \times \{1(-60 + 32) + 2(0 - 30)\}$ = -(-28 - 60) = 88End of Solution Q.3 The static indeterminacy of two span continuous beam with internal hinge is _____. Internal Hinge Sol. Internal Hinge Number of member, m = 4Number of external reaction, $r_e = 4$ Number of joint, j = 5 Number of reaction released, $r_r = 1$ Degree of static indeterminacy, $D_s = 3 m + r_e - 3j - r_r$ $= 3 \times 4 + 4 - 3 \times 5 - 1$ = 0 End of Solution

Q.4 A plane blow leak velocity component $u = \frac{x}{T_1}$, $v = -\frac{y}{T_2}$ and w = 0 along x, y and z direction respectively where $T_1 \neq 0$, $T_2 \neq 0$ are constants having dimension of time. The given blow is incompressible if





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Q.7	Modulus of elasticity of concrete	is calculated (as per IS 456 : 2000) b	v
-0	(a) Secent modulus	(b) Tangent modulus	- 5
	(c) Initial tangent modulus	(d) None of these	
		(u) None of these	
Ans.	(a)	• • • End of Solut	ion
Q.8	The flexural tensile strength of IS:456-2000, is	M 25 grade of concrete in N/mm ² , as j	
Sol.			
501.	Flexural strengt	$n = 0.7\sqrt{f_{ck}}$	
		$= 3.5 \text{ N/mm}^2$	
		End of Solut	ion
			ION
Q.9	Survey which is conducted for geo building, cities etc. is denoted a	logical features like river, natural resourc s	ces,
	(a) Land survey	(b) Geological survey	
	(c) Engineering survey	(d) Topographical survey	
Ans.			
11115.	(a)		
		• • End of Solut	ion
Q.10	The integrating factor for the d	ifferential equation $\frac{dP}{dt} + k_2P = k_1L_0e^{-k_tt}$	is
	(a) $e^{-k_1 t}$	(b) $e^{-k_2 t}$	
	(c) $e^{k_1 t}$		
		(d) $e^{k_2 t}$	
Ans.			
	I.P	$e_{\cdot} = e^{\int k_2 dt}$	
		End of Solut	ion
Q .11			
Sol.			
501.	Polar moment of inertia, I	$I_{p} = I_{x} + I_{y} = \frac{bd^{3}}{12} + \frac{db^{3}}{12}$	
		$= \frac{bd}{12}(b^2 + d^2) = \frac{2 \times 6}{12}(2^2 + 6^2) = 40$ c	-m4
		$= \frac{12}{12}(2 + 3) = \frac{12}{12}(2 + 3) = 4000$,111

Q.12 The average spacing between vehicles in a traffic stream is 50 m, then the density (in veh/km) of stream is _____.

 \Rightarrow

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Capacity =
$$\frac{1000 \times V}{S} = V \times \text{density}$$

Density = $\frac{1000}{S}$ = 20 veh/km

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Q.13 A fair (unbiased) coin was tossed 4-times in a succession and resulted in the following outcomes (I) H (II) H (III) H (IV) H. The probability of obtaining a "TAIL' when the coin is tossed again is

 $\mathbf{2}$

(a) 0 (b)
$$\frac{1}{2}$$

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(c)
$$\frac{4}{5}$$
 (d) $\frac{1}{5}$

Ans. (b)

...

$$P(E) = \frac{n(E)}{n(S)}$$

$$n(s) = [\{H\}, \{T\}] =$$

$$n(E) = \{(T)\} = 1$$

$$P(E) = \frac{1}{2}$$

End of Solution

Ans. (a)

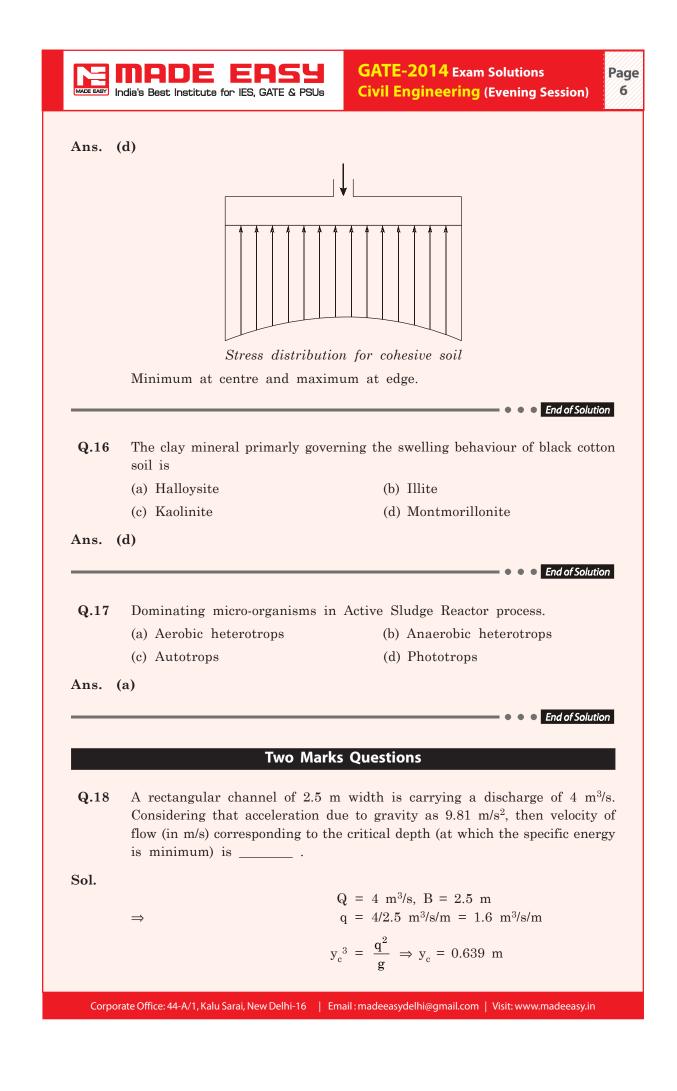
• • • End of Solution

Q.15 The contact pressure for a rigid footing resting on clay at the centre and the edges are respectively

- (a) maximum and zero
- (b) maximum and minimum
- (c) zero and maximum
- (d) minimum and maximum

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Page 5





Page

7

At critical depth velocity head = $\frac{y_c}{2}$ $\Rightarrow \qquad \frac{V^2}{2g} = \frac{0.639}{2}$ $\Rightarrow \qquad V = \sqrt{0.639 \times g}$ = 2.504 m/s

Q.19 A waste water stream (flow = 2 m³/s, ultimate BOD = 90 mg/l) is joining a small river (flow = 12 m³/s, ultimate BOD = 5 mg/l). Both water streams get mixed up instantaneously. Cross-section area of the river is 50 m². Assuming the de-oxygenation rate constant, k' = 0.25/day. The BOD (in mg/l) of the river water 10 km downstream of the mixing point is

(a)	1.68	(b) 2.63
(c)	15 46	(d) 137

Ans. (c)

(C)					
F	'low of waste water stream,	\mathbf{Q}_{w}	=	2 m ³ /sec	
U	Jltimate BOD,	$\mathbf{Y}_{\mathbf{w}}$	=	$90 \text{ mg/}l = 90 \text{ gm/m}^3$	
F	'low of river,	\mathbf{Q}_{R}	=	12 m ³ /sec	
U	Iltimate BOD of river,				
		\mathbf{Y}_{R}	=	$0.5 \text{ mg/}l = 5 \text{ gm/m}^3$	
В	30D of mixture,	\mathbf{Y}_{0}	=	$\frac{(2 \times 90) + (12 \times 5)}{2 + 12}$	
			=	$\frac{180+60}{14} = \frac{240}{14} = 17.143 \text{ gm/m}^3$	
		k_D	=	$0.434 \text{ K} = 0.434 \times 0.25 = 0.1085$	
		Y _t	=	$Y_0 \left[1 - (10)^{-k_D t} \right]$	(i)
	Area of ri	ver	=	50 m^2	
	Flow of ri	ver	=	12 m ³ /sec	
	Stream veloc	city	=	$\frac{14}{50} = 0.28 \text{ m/sec}$	
Т	'ime taken,	t	=	$\frac{10\text{km}}{0.28\text{m/s}} = 9.921\text{days}$	
	. From eq. (i)	Y _t	=	$17.143 \Big[1 - (10)^{-0.1085 \times 9.921} \Big]$	
			=	15.70 gm/m ³ = 15.70 mg/lit	
				• • • End of So	lution



Q.20 The beam of an overall depth 250 mm (shown below) is used in a building subjected to two different thermal environments. The temperature at the top and the bottom surfaces of the beam are 36°C and 72°C respectively. Considering coefficient of thermal expansion (a) as 1.50×10^{-5} /°C, the vertical deflection of the beam (in mm) at its mid span due to temperature gradient is ____ . $36^{\circ}\mathrm{C}$ 250 mm72°C 1.5 m 1.5 m Sol. L/2σ $R = \frac{h}{\alpha T}$ From properties of circle $(2R - \delta)\delta = \frac{L}{2} \times \frac{L}{2}$ (Considering ' δ ' very small so neglect δ^2) $\delta = \frac{L^2}{8R} = \frac{L^2 \alpha T}{8h}$ \Rightarrow 36°C 250 mm $72^{\circ}\mathrm{C}$ 1.5 m 1.5 mCorporate Office: 44-A/1, Kalu Sarai, New Delhi-16 | Email : madeeasydelhi@gmail.com | Visit: www.madeeasy.in

	Given,	$\alpha = 1.50 \times 10^{-5/\circ} \mathrm{C}$	
	\Rightarrow	$\delta = \frac{\alpha T L^2}{8h}$	
		$= \frac{1.50 \times 10^{-5} \times (72^{\circ} - 36^{\circ}) \times 3}{8 \times (250 \times 10^{-3})}$	3^{2}
		= 2.43 mm	
		• •	End of Solution
Q.2 1	The expression $\lim_{\alpha \to 0} \frac{x^{\alpha} - 1}{\alpha}$ is equivalent.	ial to	
	(a) log x	(b) 0	
	(c) x log x	(d) ∞	
Ans. ((a)		
	$\lim_{\alpha \to 0} \frac{\mathbf{x}^{\alpha} - 1}{\alpha} \qquad \qquad$		
	Use L-Hospital Rule		
	$\lim_{\alpha \to 0} \qquad \frac{e^{\alpha \ln x} - 1}{\alpha}$		
	$\lim_{\alpha \to 0} \frac{e^{\alpha \ln x} \ln x}{1}$		
	$= \ln $	x	
		• •	• End of Solution
Q.22	Match the List-I (Soil explore strength characteristic) and sele below:	, , , , , , , , , , , , , , , , , , , ,	
	List-I	List-II	
	P. Pressure meter test (PMT)	1. Menard's method	(E _m)

- **R.** Standard penetration (SPT) 3. Skin resistance (f_c)
- S. Vane shear test (VST)
- (a) P-1, Q-3, R-2, S-4
- (c) P-2, Q-3, R-4, S-1
- Ans. (a)

- 4. Undrained cohesion (C_u)
- (b) P-1, Q-2, R-3, S-4
- (d) P-4, Q-1, R-2, S-3

• • • End of Solution



- Page 10
- **Q.23** The suspension of sand like particles in water with particles of diameter 0.10 mm and below is flowing into a settling tank at 0.10 m³/s. Assume $g = 9.81 \text{ m/s}^2$, specific gravity of particles = 2.65, and kinematic viscosity of water = $1.0105 \times 10^{-2} \text{ cm}^2/\text{s}$. The minimum surface area (in m²) required for this settling tank to remove particles of size 0.06 mm and above with 100% efficiency is _____.

Sol.

 \Rightarrow

$$V_{s} = \frac{(G_{s} - 1)gd^{2}}{18v}$$

$$= \frac{(2.65 - 1) \times 9.81 \times (6 \times 10^{-2})^{2}}{18 \times 1.0105 \times 10^{-2}}$$

$$= \frac{1.65 \times 9.81 \times 36 \times 10^{-4}}{18 \times 1.0105 \times 10^{-2} \times 10^{2}}$$

$$= 3.204 \times 10^{-3} \text{ m/s}$$

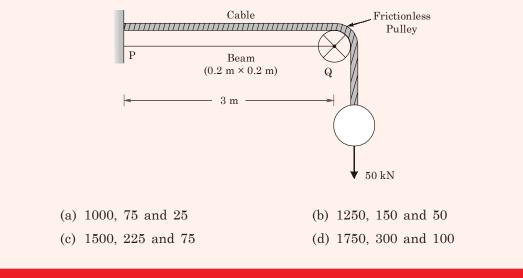
$$V_{s} = \frac{Q}{BL}$$

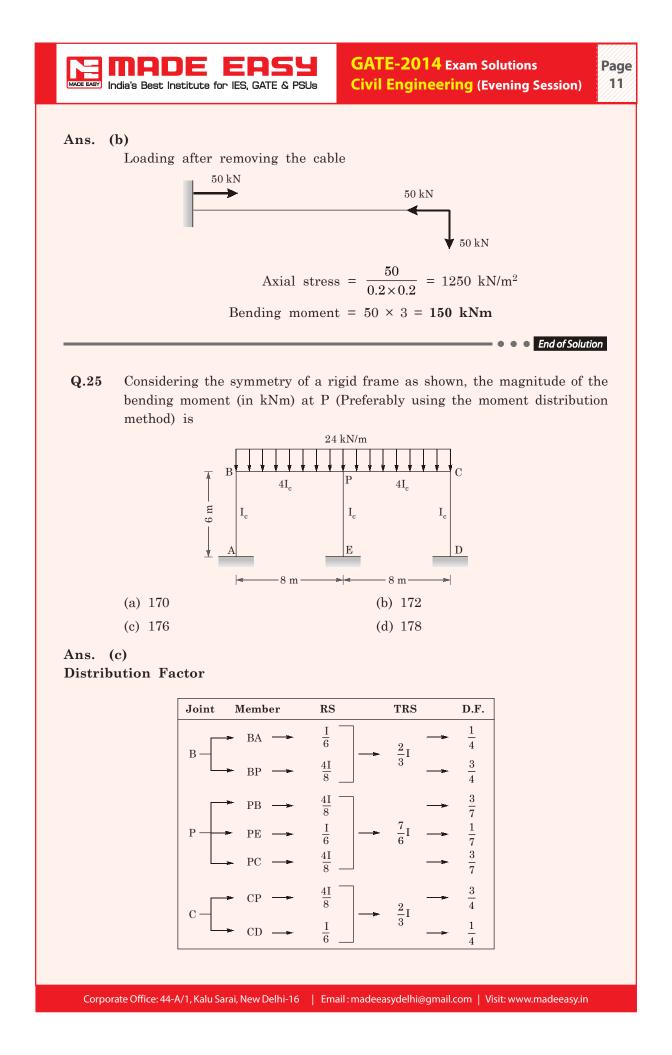
$$BL = \frac{0.1}{3.204 \times 10^{-3}}$$

$$= 31.214 \text{ m}^{2}$$

• • • End of Solution

Q.24 The values of axial stress (σ) in kN/m², bending moment (M) in kNm and shear force (V) in kN acting at point P for the arrangement shown in figure are respectively







Page 12

Fixed End Moment

$$\overline{\mathbf{M}_{AB}} = \overline{\mathbf{M}_{BA}} = \overline{\mathbf{M}_{PE}} = \overline{\mathbf{M}_{EP}} = \overline{\mathbf{M}_{CD}} = \overline{\mathbf{M}_{DC}} = 0$$

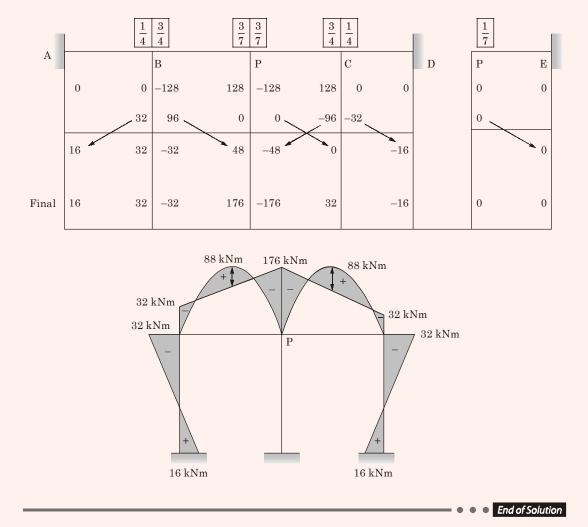
$$\overline{\mathbf{M}_{BP}} = -\frac{24 \times (8)^2}{12} = -128 \text{ kNm}$$

$$\overline{\mathbf{M}_{PB}} = +128 \text{ kNm}$$

$$\overline{\mathbf{M}_{PC}} = -128 \text{ kNm}$$

$$\overline{\mathbf{M}_{CP}} = 128 \text{ kNm}$$

Distribution Table



Q.26 An effluent at a flow rate of 2670 m³/d from a sewage treatment plant is to be disinfected. The laboratory data at disinfected studies with a chlorine dosage of 15 mg/l yield the model $N_t = N_0 e^{-0.145t}$ where $N_t =$ number of micro organism surviving at time t (in min) and $N_0 =$ number of microorganism,

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present initially (at t = 0). The volume of disinfection unit (in m^3) required to achieve a 98%. Kill of M.O. is _____.

Sol. $(50 m^3)$

 \Rightarrow

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= • • • End of Solution

Q.27 A horizontal nozzle of 30 mm diameter discharges a study Jet of water into the atmosphere at a rate of 15 litres/second. The diameter of inlet to the nozzle is 100 mm. The Jet impinges normal to a flat stationary plate held close to the nozzle end. Neglecting air friction and considering density of water as 1000 kg/m³, the force exerted by the Jet (in N) on the plate is_____ .

Sol.

Force =
$$\rho aV^2$$

 $Q = aV = 15$ litre/sec (given)
Force = $\rho \frac{Q^2}{a}$
 $= 100 \times \frac{15^2}{\frac{\pi}{4} \times 30^2} \frac{\text{kg}}{\text{m}^3} \times 10^{-6} \frac{\text{m}^6}{\text{s}^2} \times \frac{1}{10^{-6}} \text{m}^2$
 $= 100 \times \frac{1}{\pi} \text{N} = 31.83 \text{ N}$

End of Solution

Q.28 On a section of a highway the speed density relations is linear and is given by $v = \left[80 - \frac{2}{3} k \right]$; where v is in km/hr and k in vehicle/km. The capacity (in veh/hr) of this section of the highway would be

- (a) 1200 (b) 2400 (d) 9600
- (c) 4800

Ans. (b)

	$v = 80 - \frac{2k}{3}$
Capacity,	$q = v \times k$
	$= 80k - \frac{2k^2}{3}$
For q to be maximum	$\frac{\mathrm{d}\mathbf{q}}{\mathrm{d}\mathbf{k}} = 0$
\Rightarrow	$\frac{\mathrm{dq}}{\mathrm{dk}} = 80 - \frac{4\mathrm{k}}{3} = 0$

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	\Rightarrow	k = 60	
	Maximum capacity,	q = $80 \times 60 - \frac{2}{3} (60)^2$	
		$= 4800 - \frac{2}{3} \times 3600$	
		= 2400	
		• • End of Solutio	n
Q.29	content of the soil from the exact 28%. The effective root zone soil and water are 1.3 g/cm ³ a	led to a crop in a field to bring the moistur xisting 18% to the field capacity of the so of the crop is 70 cm. If the densities of th and 1 gm/cm ³ respectively, the depth of th ired for irrigating the crop is	oil ne ne
Sol.			
	Given, Root zone depth,	d = 70 cm	
		$F_{C} = 28\%$	
	Existing moisture content,		
	Density of soil, Density of water,	$\gamma = 1.3 \text{ gm/cm}^3$ $\gamma_w = 1.0 \text{ gm/cm}^3$	
	Depth of irrigation water requ		
		$d_{w} = \frac{\gamma}{\gamma_{w}} d(F_{c} - w)$	
		$= \frac{1.3}{1.0} \times (70 \times 10) (28\% - 18\%)$	
		$= 1.3 \times (70 \times 10) \times \frac{10}{100}$	
		= 91 mm	
		• • End of Solutio	n
Q.30	The rank of the matrix $\begin{bmatrix} 6\\ -2\\ 14 \end{bmatrix}$	$ \begin{bmatrix} 0 & 4 & 4 \\ 14 & 8 & 18 \\ -14 & 0 & -10 \end{bmatrix} $ is	
Sol.			
	$\begin{bmatrix} 6 & 0 & 4 & 4 \\ -2 & 14 & 8 & 18 \\ 14 & -14 & 0 & -10 \end{bmatrix}$		
	L J	$\mathbf{R}_3 \rightarrow \mathbf{R}_3 - 2\mathbf{R}_1 + \mathbf{R}_2$	

MADE India's Best Institute for IES. GATE & PSUs **Civil Engineering (Evening Session)** 15 $\begin{bmatrix} 6 & 0 & 4 & 4 \\ -2 & 14 & 8 & 18 \\ 14-2(6)+(-2) & -14-2(0)+(14) & 0-2(4)+8 & -10-2(4)+(18) \end{bmatrix}$ $\begin{bmatrix} 6 & 0 & 4 & 4 \\ -2 & 14 & 8 & 18 \\ 0 & 0 & 0 & 0 \end{bmatrix}$ Determinant of matrix $\begin{bmatrix} 6 & 0 \\ -2 & 14 \end{bmatrix}$ is not zero. \therefore Rank is 2. End of Solution

E

GATE-2014 Exam Solutions

Page

Q.31 An infinitely long slope is made up of a C-\$\$\$ soil having the properties cohesion (C) = 20 kPa and dry unit weight (γ_d) = 16 kN/m³. The angle of inclination and critical length of slope are 40° and 5 m respectively. To maintain the limiting equilibrium, the angle of internal friction of soil (in degree) is _____.

Sol.

As the given slope is in dry condition, therefore, factory of safety should be more than 2.

 $F_s = \frac{\tan \phi}{\tan b} \ge 2$ and $\beta = 40^\circ$ $tan\phi \ge 2tan (40^\circ)$ \Rightarrow $tan\phi \ge 1.6782$ \Rightarrow $\phi \geq 59.21^{\circ}$ \Rightarrow

End of Solution

Q.32 A student riding a bicycle on a 5 km one way street takes 40 minutes to reach home. The student stopped for 15 minutes during this ride. 60 vehicles over took the student (Assume the number of vehicle overtaken by the student is zero) during the ride and 45 vehicles while the student stopped. The speed of vehicle stream on that road (in km/hr) is

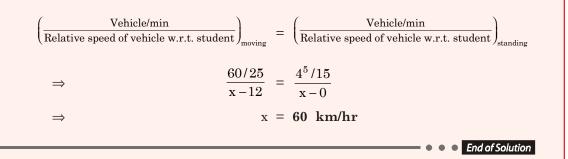
(a) 7.5 (b)	12
-------------	----

60
1

Ans. (d)

Velocity of student = $\frac{5 \text{km}}{(40-15)\text{min}} = 12 \text{km/hr}$

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Q.33 In a Marshall sample, the bulk specific gravity of mix and aggregates are 2.324 and 2.546 respectively. The sample includes 5% of bitumen (by total wt. of mix) of specific gravity 1.10. The theoretical maximum specific gravity of mix is 2.441. The void filled with bitumen (VFB) in the Marshall sample (in %) is _____.

Sol.

$$V_{v} = \frac{G_{t} - G_{m}}{G_{m}} \times 100$$

$$= \frac{2.441 - 2.324}{2.324} \times 100 = 5.03\%$$

$$V_{b} = G_{m} \frac{W_{b}}{G_{b}} = 2.324 \times \frac{5}{1.1} = 10.564$$

$$VMB = V_{v} + V_{b} = 15.594\%$$

$$VFB = \frac{V_{b} \times 100}{VMB} = \frac{10.564}{15.594} \times 100$$

$$= 67.74\%$$
End of Solution

Q.34 With reference to a standard Cartesian (x, y) plane, the parabolic velocity distribution profile of fully developed laminar flow in x-direction between two parallel plates. Stationary and identical plates that are separated by distance, h, is given by the expression

$$u = -\frac{h^2 dP}{8\mu dx} \left[1 - 4 \left(\frac{y}{h}\right)^2 \right]$$

In this equation the y = 0 axis lies equidistant between the plates at a distance h/2 from the two plates, P is the pressure variable and μ is the dynamic viscosity term, the maximum and average velocities are respectively

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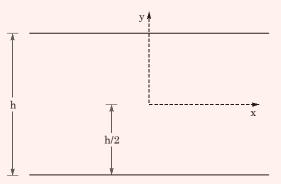
(a)
$$U_{max} = -\frac{h^2 dP}{8\mu dx}$$
 and $U_{avg} = \frac{2}{3}U_{max}$

(b)
$$U_{\text{max}} = \frac{h^2 dP}{8\mu dx}$$
 and $U_{\text{avg}} = \frac{2}{3} U_{\text{max}}$

(c)
$$U_{\text{max}} = -\frac{h^2 dP}{8\mu dx}$$
 and $U_{\text{avg}} = \frac{3}{8}U_{\text{max}}$

(d)
$$U_{\text{max}} = \frac{h^2 dP}{8\mu dx}$$
 and $U_{\text{avg}} = \frac{3}{8} U_{\text{max}}$

Ans. (a)



Velocity expression for a laminar flow between two parallel plates is

$$U = -\frac{h^2}{8\mu} \left(\frac{dP}{dx}\right) \left[1 - 4\left(\frac{y}{h}\right)^2\right]$$

End condition,

 $U = U_{max} \text{ at } y = 0$ $U_{max} = -\frac{h^2}{8\mu} \left(\frac{dP}{dx}\right)$

 \Rightarrow

arge, $dQ = Area \times Velocity$

$$dQ = \left[-\frac{h^2}{8\mu}\left(\frac{dP}{dx}\right)\left(1-4\left(\frac{y}{h}\right)^2\right)\right]\left(dy \times 1\right)$$

 \Rightarrow

 \Rightarrow

$$Q = -\frac{h^{2}}{8\mu} \left(\frac{dP}{dx}\right) \int_{-h/2}^{h/2} \left(1 - \frac{4y^{2}}{h^{2}}\right) dy$$
$$= -\frac{h^{2}}{8\mu} \left(\frac{dP}{dx}\right) \left[y - \frac{4y^{3}}{3h^{2}}\right]_{-h/2}^{h/2}$$

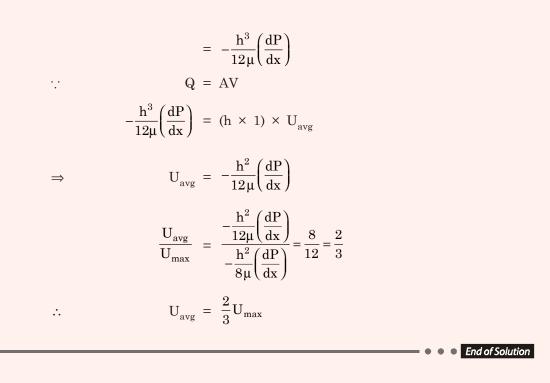
$$= -\frac{h^2}{8\mu} \left(\frac{dP}{dx}\right) \left[\left\{ \frac{h}{2} - \left(-\frac{h}{2}\right) \right\} - \left\{ \frac{4}{3h^2} \left(\frac{h^3}{8} - \left(-\frac{h^3}{8}\right) \right) \right\} \right]$$

- >

Page 17

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Page 18



Q.35 A venturimeter having a throat diameter of 0.1 m is used to estimate the flow rate of a horizontal pipe having a dia of 0.2 m for an observed pressure difference of 2 m of water head and coefficient of discharge equal to unity, assuming that the energy losses are negligible. The flow rate (in m^3/s) through the pipe is approximately equal to

(a)	0.500	(b)	0.150
(c)	0.050	(d)	0.015

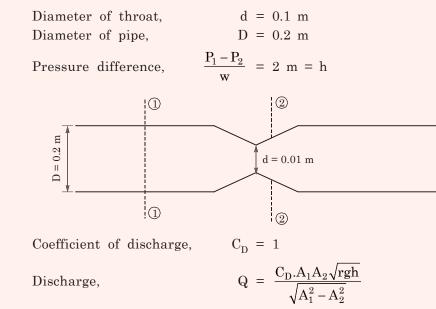
(c)	0.050	(d)	0.01

Ans. (c)

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 \Rightarrow

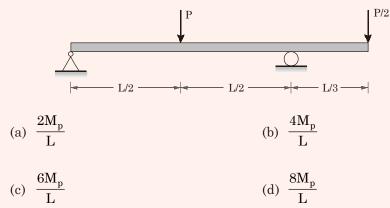
$$A_{1} = \frac{\pi}{4} (0.2)^{2}$$

$$A_{2} = \frac{\pi}{4} (0.1)^{2}$$

$$Q = \frac{1 \times \left(\frac{\pi}{4}\right)^{2} \times (0.2)^{2} (0.1)^{2} \sqrt{2 \times 9.81 \times 2}}{\left(\frac{\pi}{4}\right) \sqrt{(0.2)^{4} - (0.1)^{4}}}$$

$$= 0.0508 \approx 0.050 \text{ m}^{3}/\text{sec}$$
Endof Solution

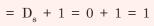
Q.36 A prismatic beam (shown) has plastic moment capacity of M_p , then the collapse lead P of the beam is

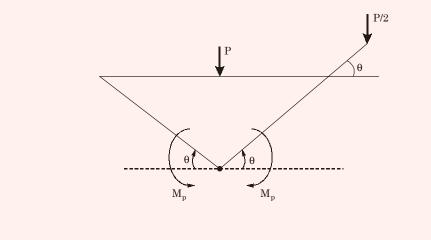


Ans. (c)

Here degree of static indeterminacy = 0

: Number of plastic hinges required for mechanical





From principal of virtual work

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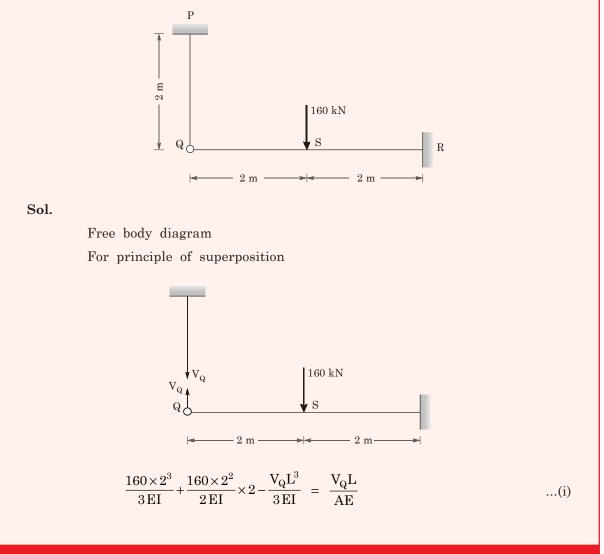
$$-M_{p}\theta - M_{p}\theta + P\frac{L}{2}\theta - \frac{P}{2} \times \frac{L}{3}\theta = 0$$

$$\Rightarrow \qquad -2M_{p}\theta + \frac{PL}{2}\theta - \frac{PL}{6}\theta = 0$$

$$\Rightarrow \qquad 2M_{p} = \frac{PL}{2} \times \frac{PL}{6} = \frac{(3-1)PL}{6} = \frac{1}{3}PL$$

$$\Rightarrow \qquad P = \frac{6Mp}{L}$$

Q.37 The axial load (in kN) in the member PQ for the assembly/arrangement shown in figure given below is _____.



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Page 20

End of Solution

Deflections due to axial forces will be very less as compared to bending forces.

So we can neglect the axial deformation.

EA

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 \therefore From equation (i)

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$$\frac{160 \times 2^3}{3 \,\text{EI}} + \frac{160 \times 2^2}{2 \,\text{EI}} \times 2 - \frac{V_Q 4^3}{3 \,\text{EI}} = 0$$

$$\Rightarrow \qquad \frac{160 \times 2^3}{3} + \frac{160 \times 2^2 \times 2}{2} = \frac{V_Q \times 4^3}{3}$$
$$\Rightarrow \qquad V_Q = 50 \text{ kN}$$

Q.38 Water is flowing at a steady rate through a homogeneous and saturated horizontal soil strip at 10 meter length. The strip is being subjected to a constant water head (H) of 5 m at the beginning and 1 m at the end. If the

governing equation of the flow in the soil strip is $\frac{d^2H}{dx^2} = 0$ (where x is the distance along the soil strip), the value of H (in m) at the middle of the strip is ______ .

Sol.

Given, equation of the flow of soil strip is

	$\frac{d^2H}{dx^2} = 0$
\Rightarrow	$\frac{dH}{dx} = C_1$
\Rightarrow at x = 0, H = 5 m	$\mathbf{H} = \mathbf{C}_1 \mathbf{x} + \mathbf{C}_2$
at $x = 0$, $\Pi = 0$ $\Pi \Rightarrow$	$C_2 = 5$
\Rightarrow	$\dot{H} = C_1 x + 5$
at $x = 10$, $H = 1$ m	
\Rightarrow	$1 = C_1 \times 10 + 5$
\Rightarrow \Rightarrow	$10C_1 = -4$
\Rightarrow	$C_1 = -\frac{2}{5}$
\Rightarrow	$H = -\frac{2}{5}x + 5$

EXAMPLE 2014 Exam Solutions **Civil Engineering (Evening Session)** $\begin{array}{l} \text{Page} \\ \text{Descent for IES, GATE & PSUs} \\
\begin{array}{l} \text{At } x = 5 \ \text{m} \\
\end{array}$ $\begin{array}{l} \text{H} = -\frac{2}{5} \times 5 + 5 \\
\text{H} = 3 \ \text{m} \\
\end{array}$ $\begin{array}{l} \text{End of Solution} \\
\end{array}$

Q.39 An observer counts 240 veh/hr at a specific highway location. Assume that the vehicle arrival at the location is Poisson distributed, the probability of having one vehicle arriving over 30 sec time interval is _____.

Sol.

$$P(n, t) = \frac{e^{-\lambda t} . (\lambda t)^n}{n!}$$

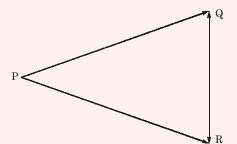
 $\lambda = no. \text{ of vehicles} = 240 \text{ vehicle/km}$

Here,

$$P(1, 30) = \frac{e^{\frac{-240\times30}{3600}} \left(\frac{240}{3600} \times 30\right)^{1}}{1!}$$
$$= 2.e^{-2}$$
$$= 0.2707$$

End of Solution

Q.40 A tachometer was placed at point P to estimate the horizontal distance PQ and PR. The corresponding stadia intercept with the telescope kept horizontal are 0.320 and 0.210 m respectively. The ∠QPR is measured to be 61°30′30″. If the stadia multiplication constant = 100 and stadia addition constant = 0.10 m, the horizontal distance (in m) between the point Q and R is _____.



Sol.

PQ = ks + C= 100(0.32) + 0.1 = 32.1 m PR = ks + C= 100(0.21) + 0.1 = 21.1 m



Applying the cosine rule

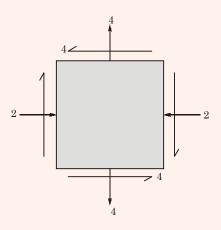
$$QR = \sqrt{PQ^2 + PR^2 + 2(PQ)(PR)\cos\theta}$$

Where, $\theta = 61^{\circ}30'30'' = 61.508^{\circ}$

$$= \sqrt{1030.41 + 445.21 + 2(32.1)(21.1)\cos 61.508^{\circ}}$$

= $\sqrt{1030.41 + 445.21 + 696.2}$
= 46.06 m
End of Solution

Q.41 For the state of stresses (in MPa) shown in the figure below, the maximum shear stress (in MPa) is



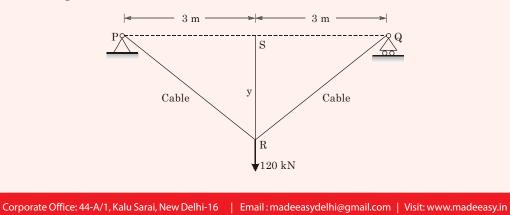
Sol.

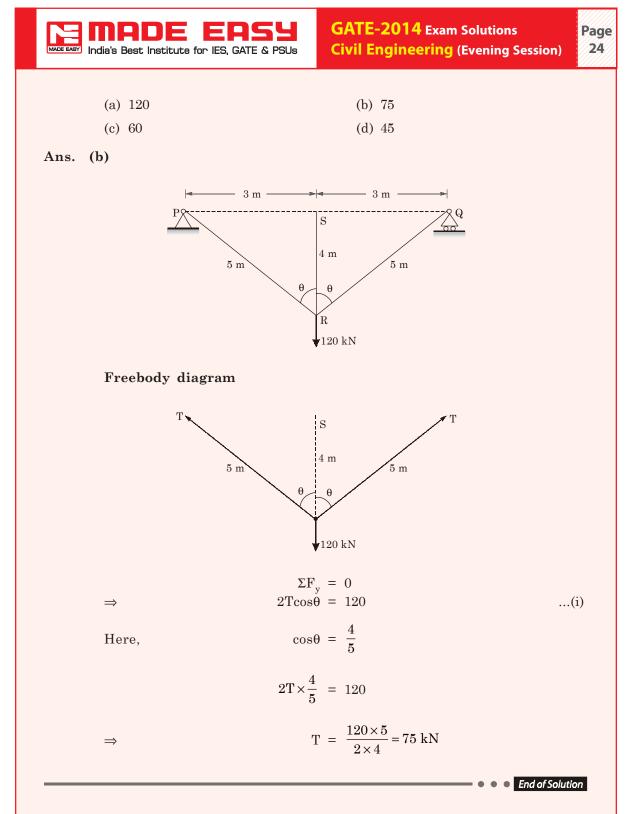
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$$\tau_{\max} = \frac{\sigma_1 - \sigma_2}{2} = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right) + \tau_{xy}^2}$$
$$= 5.0 \text{ MPa}$$

End of Solution

Q.42 The tension (in kN) in a 10 m long cable shown in figure neglecting its self weight is





Q.43 A pre-timed four phase signal has critical lane flow rate for the first three phases as 200, 187 and 210 veh/hr with saturation flow rate of 1800 veh/hr/lane for all phases. The lost time is given as 4 seconds for each phase. If the cycle length is 60 seconds. The efficiency gree time of 4th phase is ______ (in seconds).

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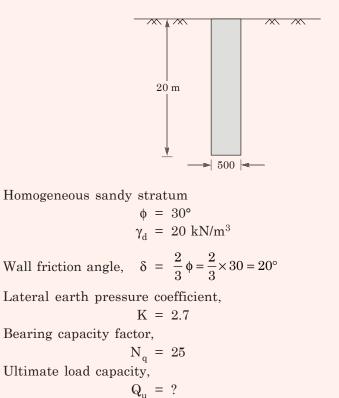
Page 25

Flow rates for the first	three phase are given as
	$q_1 = 200$ veh/hr
	$q_2 = 187$ veh/hr
and	$q_3 = 210$ veh/hr
Saturation flow rate is 1	800 veh/hr/lane
Lost time,	$L = 4 \times 4 = 16 \text{ sec}$
length of the cycle,	$C_0 = 60 \text{ sec}$
Now,	$y_1 = \frac{q_1}{s_1} = \frac{200}{1800}$
	$y_2 = \frac{q_2}{s_2} = \frac{187}{1800}$
	$y_3 = \frac{q_3}{s_3} = \frac{210}{1800}$
	$C_0 = \frac{1.5L + 5}{1 - y}$
⇒	$60 = \frac{1.5 \times 16 + 5}{1 - y}$
\Rightarrow	$60 = \frac{24+5}{1-y}$
⇒	$1 - y = \frac{29}{60}$
⇒	$y = \frac{1-29}{60} = \frac{60-29}{60}$
And	$= 0.517 y = y_1 + y_2 + y_3 + y_4$
	$0.517 = \frac{597}{1800} + y_4$
\Rightarrow	$0.517 = \frac{1}{1800} + y_4$
\Rightarrow	$y_4 = 0.185$
	$G = \frac{y_4}{y} (C_0 - L)$
	$= \frac{0.185}{0.517} (60 - 16)$
	= 15.745 sec



Q.44 A single vertical friction pile of dia 500 mm and length 20 m is subjected to a vertical compressive load. The pile is embedded in a homogeneous sandy stratum where; angle of internal friction (ϕ) = 30°, dry unit weight $(\gamma_d) = 20 \text{ kN/m}^3$ and angle of wall friction (δ) = 2 ϕ /3. Considering the coefficient of lateral earth pressure (k) = 2.7 and the bearing capacity factor $(N_{o}) = 25$, the ultimate bearing capacity of the pile is _

Sol.



Vertical effective stress at 20 m,

$$\overline{\sigma_{\rm w}} = 20 \times 20 = 400 \text{ kN/m}^2$$

From 0 to 20 m, unit point bearing resistance and skin friction resistance remain constant at

$$\overline{\sigma_{\rm v}}$$
 = 400 kN/m²

The ultimate load capacity is given by

$$\begin{array}{rcl} \mathbf{Q}_{\mathrm{u}} &=& \mathbf{q}_{\mathrm{up}} \cdot \mathbf{A}_{\mathrm{p}} + \mathbf{q}_{\mathrm{s}} \cdot \mathbf{A}_{\mathrm{s}} \\ &=& \mathbf{Q}_{\mathrm{up}} + \mathbf{f}_{\mathrm{s}} \end{array}$$

where

$$= Q_{up} + f_s$$

$$q_{up} = \overline{\sigma_v} N_q$$

$$= 400 \times 25$$

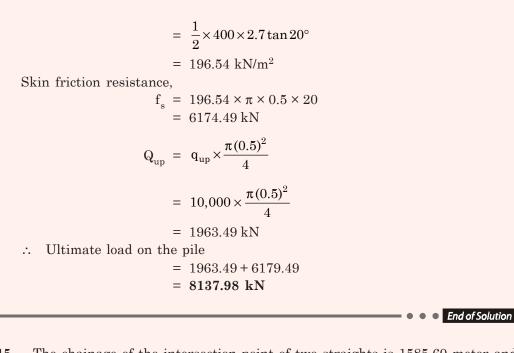
$$= 10000 \text{ kN/m}^2$$

$$q_s = \frac{1}{2} \cdot \overline{\sigma_v} \cdot \text{K} \cdot \tan \delta$$

and



Page 27

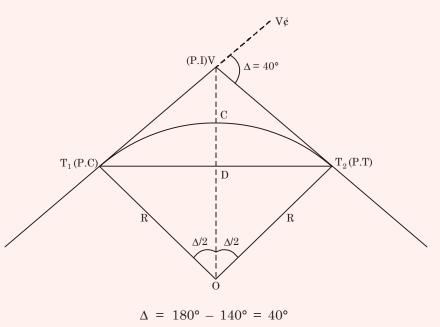


Q.45 The chainage of the intersection point of two straights is 1585.60 meter and the angle of inter section is 140°. If the radius of a circular curves is 600 meter, the tangent distance (in m) and length of the curve (in m) respectively are

(c) 218.38 and 418.88

(b) 218.38 and 1648.49(d) 418.88 and 218.38





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Page 28

Length of the curve = $\frac{\pi R}{180^{\circ}}\Delta$

$$= \frac{\pi \times 600}{180} \times 40 = 418.82 \,\mathrm{m}$$

Tangent distance (T) is the distance between P-C to P.I (also the distance from P.I to P.T)

 \Rightarrow

$$T = T_1 V = T_2 V = OT_1 \tan \frac{\Delta}{2} = R \tan \frac{\Delta}{2}$$
$$= 600 \tan 20^\circ$$
$$= 218.88 \text{ m}$$

• • • End of Solution

Q.46 A surface water treatment plant operates round the clock with a flaw rate of $35 \text{ m}^3/\text{min}$. The water temperature is 15°C and Jar testing indicated and alum dosage of 25 mg/l with flocculation at a Gt value of 4×10^4 producing optimal results. The alum quantity required for 30 days (in kg) of operation of the plant is ______ .

Sol.

Given data		
Flow rate,	Q	= 35 m ³ /min
	Gt	$= 4 \times 10^4$
	Alum dosage	= 25 mg/lit.
Alum quantity	(kg) required	for 30 days
		$= 35 \times 10^3 \times 60 \times 24 \times 30 \times 25 \times 10^{-6}$
		= 37,800 kg

• • • End of Solution

Section - II (General Aptitude)

One Mark Questions

Q.47 The population of a new city is 5 million and is growing at a rate of 20% annually. How many year would it take to double at this growth rate.

(a) 3-4 year(c) 5-6 year

(b) 4-5 year(d) 6-7 year

