



# MADE EASY

India's Best Institute for IES, GATE & PSUs

# GATE 2018

## Civil Engineering

Date of Test 11-02-2018 : Shift -2

### Detailed Solutions

- MADE EASY has taken due care in making solutions. If you find any discrepancy/typo/technical error, kindly share/post your views.
- If you want to contest the answer key given by MADE EASY, kindly post your suggested answer with detailed explanations at [www.madeeasy.in](http://www.madeeasy.in)
- Students are requested to share their expected marks in GATE 2018.

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**GENERAL APTITUDE**

- Q.1** A three-member committee has to be formed from a group of 9 people. How many such distinct committees can be formed?  
(a) 27 (b) 72  
(c) 81 (d) 84

**Ans. (d)**

$${}^9C_3 = \frac{9!}{6! \times 3!} = \frac{9 \times 8 \times 7}{6} = 84$$

● ● ● **End of Solution**

- Q.2** "His face \_\_\_\_\_ with joy when the solution of the puzzle was \_\_\_\_\_ to him."  
The words that best fill the blanks in the above sentence are  
(a) shone, shown (b) shone, shone  
(c) shown, shone (d) shown, shown

**Ans. (a)**

Shone - It is past - participle and past form of shine.  
Shown - To show means to reveal and point out something.

● ● ● **End of Solution**

- Q.3** For non-negative integers,  $a, b, c$ , what would be the value of  $a + b + c$  if  $\log a + \log b + \log c = 0$ ?  
(a) 3 (b) 1  
(c) 0 (d) -1

**Ans. (a)**

As  $a, b, c$  are non-negative integers and given  $\log a + \log b + \log c = 0$

$$\log(a \times b \times c) = \log 1$$

$$\Rightarrow a \times b \times c = 1$$

Which can be possible for simple values.

$$a = b = c = 1$$

$$\text{Hence } a + b + c = 1 + 1 + 1 = 3$$

**Alternate Method:**

Given,

$$\log a + \log b + \log c = 0$$

As we know  $\log 1 = 0$ , so each one of them can be zero if  $a = b = c = 1$

$$\log 1 + \log 1 + \log 1 = 0$$

By putting  $a = b = 1$  equation satisfies

$$a + b + c = 1 + 1 + 1 = 3$$

● ● ● **End of Solution**

**Q.4** "Although it does contain some pioneering ideas, one would hardly characterize the work as \_\_\_\_\_."

The word that best fills the blank in the above sentence is

- (a) innovative (b) simple  
(c) dull (d) boring

**Ans. (a)**

Innovative is similar to pioneer.

● ● ● End of Solution

**Q.5**  $\underbrace{a+a+a+\dots+a}_{n \text{ times}} = a^2b$  and  $\underbrace{b+b+b+\dots+b}_{m \text{ times}} = ab^2$ , where  $a, b, n$  and  $m$  are natural

numbers. What is the value of  $\left(\frac{m+m+m+\dots+m}{n \text{ times}}\right)\left(\frac{n+n+n+\dots+n}{m \text{ times}}\right)$ ?

- (a)  $2a^2b^2$  (b)  $a^4b^4$   
(c)  $ab(a+b)$  (d)  $a^2 + b^2$

**Ans. (b)**

$$\frac{a+a+a+\dots+a}{n \text{ times}} = na = a^2b$$

$$\Rightarrow n = ab \quad \dots(i)$$

$$\frac{b+b+b+\dots+b}{m \text{ times}} = mb = b^2a$$

$$\Rightarrow m = ab \quad \dots(ii)$$

$$\text{So, } \frac{[m+m+\dots+m]}{n \text{ times}} \times \frac{[n+n+\dots+n]}{m \text{ times}}$$

$$\text{i.e., } mn \times mn = (mn)^2$$

$$\text{from (i) and (ii) } mn = a^2b^2$$

$$\text{So, result, } (mn)^2 = (a^2b^2) = a^4b^4$$

● ● ● End of Solution

**Q.6** In manufacturing industries, loss is usually taken to be proportional to the square of the deviation from a target. If the loss is Rs. 4900 for a deviation of 7 units, what would be the loss in Rupees for a deviation of 4 units from the target?

- (a) 400 (b) 1200  
(c) 1600 (d) 2800

**Ans. (c)**

$$\text{Loss} = kd^2 \quad \text{For duration of 7 units}$$

$$4900 = k(7)^2 \Rightarrow k = 100$$

$$\text{Loss} = kd^2 \quad \text{For duration of 4 units}$$

$$= k(4)^2 \Rightarrow 16k = 1600$$

● ● ● End of Solution





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**CIVIL ENGINEERING**

**Q.1** As per IS 456 : 2000, the minimum percentage of tension reinforcement (up to two decimal places) required in reinforced-concrete beams of rectangular cross-section (considering effective depth in the calculation of area) using Fe500 grade steel is \_\_\_\_\_.

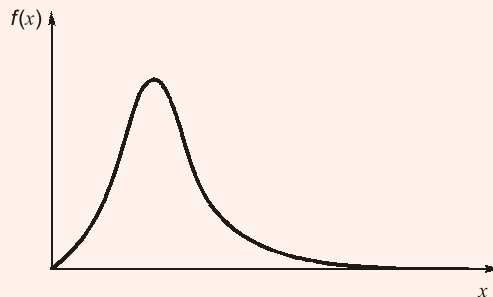
**Ans. (0.17)**

Minimum percentage of steel (for Fe500)

$$= \frac{85}{f_y} \% = \frac{85}{500} \% = 0.17\%$$

● ● ● End of Solution

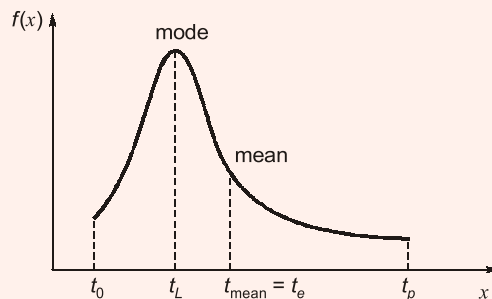
**Q.2** A probability distribution with right skew is shown in the figure.



The correct statement for the probability distribution is

- (a) Mean is equal to mode
- (b) Mean is greater than median but less than mode
- (c) Mean is greater than median and mode
- (d) Mode is greater than median

**Ans. (c)**



$$t_L < t_{\text{mean}} = \text{Curve is skew to right.}$$

$$\text{mode} < \text{mean}$$

i.e., mean > median and mode

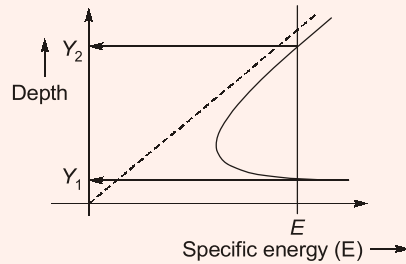
Mean is greater than the mode and the median. This is common for a distribution that is skewed to the right [i.e., bunched up toward the left and a 'tail' stretching toward the right].

● ● ● End of Solution

- Q.3** For a given discharge in an open channel, there are two depths which have the same specific energy. These two depths are known as
- (a) alternate depths (b) critical depths  
(c) normal depths (d) sequent depths

**Ans. (a)**

Depth with same specific energy are called Alternate depths of flow. It represents a subcritical depth of flow ( $Y_2$ ) and a supercritical depth of flow ( $Y_1$ ).



● ● ● End of Solution

- Q.4** Peak Hour Factor (PHF) is used to represent the proportion of peak sub-hourly traffic flow within the peak hour. If 15-minute sub-hours are considered, the theoretically possible range of PHF will be
- (a) 0 to 1.0 (b) 0.25 to 0.75  
(c) 0.25 to 1.0 (d) 0.5 to 1.0

**Ans. (c)**

$$0.25 \leq \text{PHF}_{15} \leq 1$$

● ● ● End of Solution

- Q.5** The solution of the equation  $x \frac{dy}{dx} + y = 0$  passing through the point (1, 1) is
- (a)  $x$  (b)  $x^2$   
(c)  $x^{-1}$  (d)  $x^{-2}$

**Ans. (c)**

$$x \frac{dy}{dx} + y = 0$$

$$x \frac{dy}{dx} = -y$$

$$\frac{dy}{y} = -\frac{dx}{x}$$

$$\int \frac{1}{y} dy = \int \frac{-1}{x} dx$$

$$\ln y = -\ln x + c$$

when  $y = 1, x = 1$

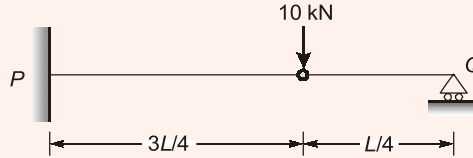
$$c = 0$$

$$\Rightarrow y = \frac{1}{x} = x^{-1}$$

● ● ● End of Solution



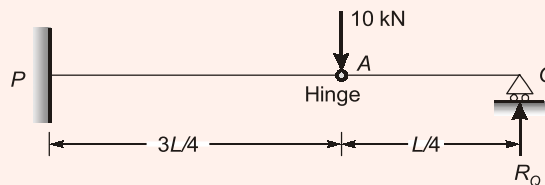
- Q.6** A vertical load of 10 kN acts on a hinge located at a distance of  $L/4$  from the roller support  $Q$  of a beam of length  $L$  (see figure).



The vertical reaction at support  $Q$  is

- (a) 0.0 kN (b) 2.5 kN  
(c) 7.5 kN (d) 10.0 kN

**Ans. (a)**



Bending moment about hinge point  $A = 0$  (consider the right hand side of  $A$ )

$$R_Q \times \frac{L}{4} = 0$$

$$R_Q = 0 \text{ kN}$$

• • • **End of Solution**

- Q.7** Which one of the following statements is NOT correct?
- (a) When the water content of soil lies between its liquid limit and plastic limit, the soil is said to be in plastic state.  
(b) Boussinesq's theory is used for the analysis of stratified soil.  
(c) The inclination of stable slope in cohesive soil can be greater than its angle of internal friction.  
(d) For saturated dense fine sand, after applying overburden correction, if the Standard Penetration Test value exceeds 15, dilatancy correction is to be applied.

**Ans. (b)**

Boussinesq's assumed soil as isotropic hence not applicable for stratified soil.

• • • **End of Solution**

- Q.8** A reinforced-concrete slab with effective depth of 80 mm is simply supported at two opposite ends on 230 mm thick masonry walls. The centre-to-centre distance between the walls is 3.3 m. As per IS 456 : 2000, the effective span of the slab (in m, up to two decimal places) is \_\_\_\_\_.

Ans. (3.15)

Effective depth

$$d = 80 \text{ mm}$$

$$\text{Width of support} = 230 \text{ mm}$$

$$\text{c/c distance between walls} = 3.30 \text{ m}$$

$$\text{Clear span of slab} = 3.30 - 0.23 = 3.07 \text{ m}$$

$$\text{Effective span} = \text{Minimum} \begin{cases} \bullet (L_{\text{clear}} + d) \\ \bullet \text{c/c distance between supports} \end{cases}$$

$$= \text{Minimum} \begin{cases} \bullet (3.07 + 0.08 = 3.15 \text{ m}) \\ \bullet 3.3 \text{ m} \end{cases}$$

So,  $L_{\text{eff}} = 3.15 \text{ m}$

● ● ● End of Solution

- Q.9 As per IS 10500:2012, for drinking water in the absence of alternate source of water, the permissible limits for chloride and sulphate, in mg/L, respectively are
- (a) 250 and 200 (b) 1000 and 400  
(c) 200 and 250 (d) 500 and 1000

Ans. (b)

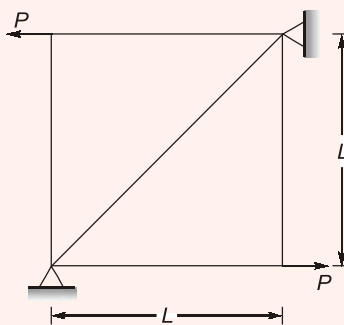
As per IS-10500 : 2012

Permissible limit in absence of alternate source.

Chloride (as Cl <sup>-</sup> )	Sulphate as [SO <sub>4</sub> ] <sup>-2</sup>
1000	400

● ● ● End of Solution

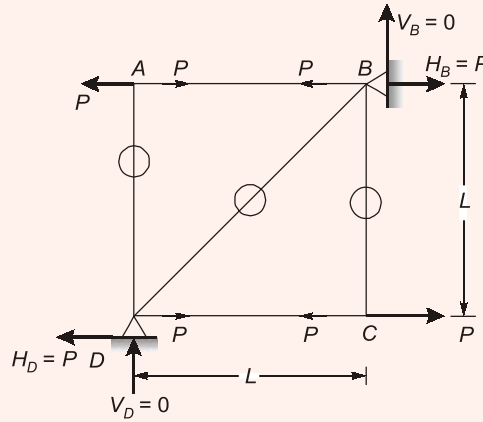
- Q.10 All the members of the planar truss (see figure), have the same properties in terms of area of cross-section (A) and modulus of elasticity (E),



For the loads shown on the truss, the statement that correctly represents the nature of forces in the members of the truss is:

- (a) There are 3 members in tension, and 2 members in compression  
(b) There are 2 members in tension, 2 members in compression, and 1 zero-force member  
(c) There are 2 members in tension, 1 member in compression, and 2 zero-force members  
(d) There are 2 members in tension, and 3 zero-force Members

Ans. (d)



Since member  $BD$  neither elongate nor contract.

Hence,  $F_{BD} = 0$

So there are 2 tension members ( $AB$  and  $DC$ ) and 3 zero force members ( $AD$ ,  $BD$ ,  $BC$ ).

● ● ● End of Solution

- Q.11 The initial concavity in the Load-penetration curve of a CBR test is NOT due to
- (a) uneven top surface (b) high impact at start of loading  
(c) inclined penetration plunger (d) soft top layer of soaked soil

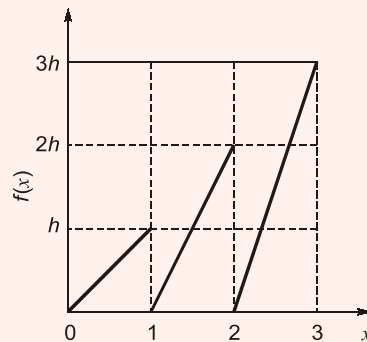
Ans. (b)

Initial concavity in CBR test due to :

- Improper compaction
- Soft top layer
- Inclined plunger

● ● ● End of Solution

- Q.12 The graph of a function  $f(x)$  is shown in the figure.



For  $f(x)$  to be valid probability density function, the value of  $h$  is

- (a)  $\frac{1}{3}$  (b)  $\frac{2}{3}$   
(c) 1 (d) 3

Ans. (a)

$$\int_0^3 f(x) dx = 1$$

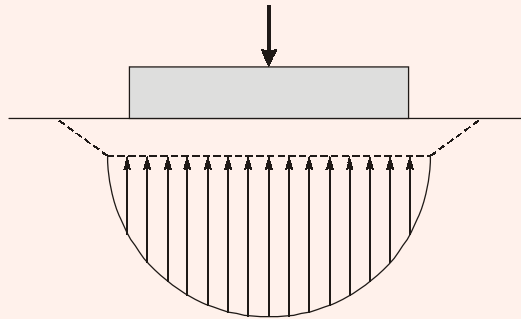
$$\int_0^1 f(x) dx + \int_1^2 f(x) dx + \int_2^3 f(x) dx = 1$$

$$\frac{h}{2} + \frac{2h}{2} + \frac{3h}{2} = 1$$

$$6h = 2 \Rightarrow h = \frac{1}{3}$$

● ● ● End of Solution

Q.13 The contact pressure and settlement distribution for a footing are shown in the figure.



The figure corresponds to a

- (a) rigid footing on granular soil      (b) flexible footing on granular soil  
(c) flexible footing on saturated clay      (d) rigid footing on cohesive soil

Ans. (a)

Rigid footing on granular soil.

● ● ● End of Solution

Q.14 The clay mineral, whose structural units are held together by potassium bond is

- (a) Halloysite      (b) Illite  
(c) Kaolinite      (d) Smectite

Ans. (b)

Kaolinite → Hydrogen bond  
Illite → Ionic Bond/Potassium Bond  
Montmorillonite → Water Bond

● ● ● End of Solution

Q.15 A fillet weld is simultaneously subjected to factored normal and shear stresses of 120 MPa and 50 MPa, respectively. As per IS 800 : 2007, the equivalent stress (in MPa, up to two decimal places) is

**Ans. (147.99)**

Factored normal stress,  $f_a = 120$  MPa

Factored shear stress,  $q = 50$  MPa

According to IS-800 : 2007, clause 10.5.10.1.1

The equivalent stress,  $f_e = \sqrt{f_a^2 + 3q^2} \leq \frac{f_u}{\sqrt{3}Y_{mw}}$

$$f_e = \sqrt{120^2 + 3 \times 50^2} = 147.99$$

$$\Rightarrow f_e \leq \frac{f_u}{\sqrt{3}y_{mw}} = \frac{400}{\sqrt{3} \times 1.25} = (184.75 \text{ MPa})$$

**Note :** The above check for combination of stresses need not be done for fillet welds where the sum of normal and shear stresses does not exceed  $f_{wd}$  [clause 10.5.10.1.2(b)]

Hence, sum of normal and shear stresses =  $120 + 50 = 170$  MPa  $\leq f_{wd}$  (= 184.75 MPa)

So, the above check need not be done.

So, weld is designed for a resultant shear stress of ( $f_r$ )

$$f_r = \sqrt{f_a^2 + q^2} = \sqrt{120^2 + 50^2} = 130 \text{ N/mm}^2$$

So, the design stress is 130 N/mm<sup>2</sup>; however equivalent stress 147.99 MPa.

● ● ● **End of Solution**

**Q.16** A structural member subjected to compression, has both translation and rotation restrained at one end, while only translation is restrained at the other end. As per IS 456 : 2000, the effective length factor recommended for design is

- (a) 0.50 (b) 0.65  
(c) 0.70 (d) 0.80

**Ans. (d)**

One end is fixed

Other end is pin jointed

Effective length of column (as per IS:456-2000)

$$= 0.80 L$$



● ● ● **End of Solution**

**Q.17** The intensity of irrigation for the Kharif season is 50% for an irrigation project with culturable command area of 50,000 hectares. The duty for the Kharif season is 1000 hectare cumec. Assuming transmission loss of 10%, the required discharge (in cumec, up to two decimal places) at the head of the canal is \_\_\_\_\_.

**Ans. (27.78)**

Culturable command area = 50000 ha

Intensity of irrigation for kharif season = 50%

∴ Area under kharif = 25000 ha

Duty for kharif season = 1000 ha/cumec

$$\text{Duty} = \frac{\text{Area}}{\text{Discharge}}$$

∴ Discharge at the head of field

$$Q = \frac{25000 \text{ ha}}{1000 \text{ ha/cumec}} = 25 \text{ cumec}$$

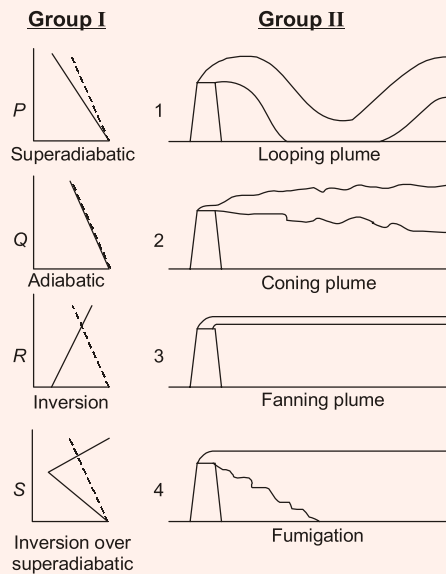
Transmission/conveyance loss = 10%

∴  $\eta_{\text{conveyance}} = 90\%$

Discharge at the head of canal =  $\frac{25}{0.9} \text{ cumec} = 27.78 \text{ cumec}$

● ● ● End of Solution

**Q.18** In the figures. Group I represents the atmospheric temperature profiles (P, Q, R and S) and Group II represents dispersion of pollutants from a smoke stack (1, 2, 3 and 4). In the figures of Group I, the dashed line represents the dry adiabatic lapse rate, whereas the horizontal axis represents temperature and the vertical axis represents the altitude.



The correct match is

- (a) P-1, Q-2, R-3, S-4  
(c) P-1, Q-4, R-3, S-2

- (b) P-1, Q-2, R-4, S-3  
(d) P-3, Q-1, R-2, S-4

**Ans. (a)**

● ● ● End of Solution

**Q.19** Probability (up to one decimal place) of consecutively picking 3 red balls without replacement from a box containing 5 red balls and 1 white ball is \_\_\_\_\_.

**Ans. (0.5)**

Probability, 
$$\bar{P} = \frac{5}{6} \times \frac{4}{5} \times \frac{3}{4} = \frac{1}{2} = 0.5$$

• • • End of Solution

**Q.20** A flownet below a dam consists of 24 equipotential drops and 7 flow channels. The difference between the upstream and downstream water levels is 6 m. The Length of the flow line adjacent to the toe of the dam at exit is 1 m. The specific gravity and void ratio of the soil below the dam are 2.70 and 0.70, respectively. The factor of safety against piping is

- (a) 1.67 (b) 2.5  
(c) 3.4 (d) 4

**Ans. (d)**

$N_f = 7 \quad N_d = 24 \quad H = 6 \text{ m}$

Critical Hydraulic Gradient  $i_c = \frac{G-1}{1+e} = \frac{2.7-1}{1+0.7} = 1$

Exit Gradient ( $i_{exit}$ ) =  $\frac{\Delta h}{l} = \frac{\left(\frac{H}{N_d}\right)}{l} = \frac{\left(\frac{6}{24}\right)}{1\text{m}} = \frac{1}{4}$

F.O.S. =  $\frac{i_c}{i_{exit}} = \frac{1}{\left(\frac{1}{4}\right)} = 4$

• • • End of Solution

**Q.21** As per IRC : 37-2012, in order to control subgrade rutting in flexible pavements, the parameter to be considered is

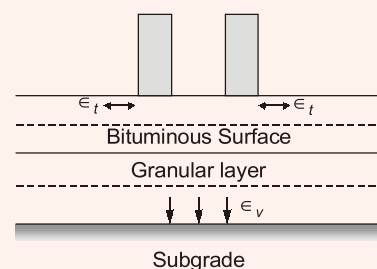
- (a) horizontal tensile strain at the bottom of bituminous layer  
(b) vertical compressive strain on top of subgrade  
(c) vertical compressive stress on top of granular layer  
(d) vertical deflection at the surface of the pavement

**Ans. (b)**

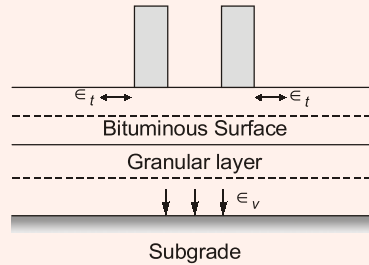
As per IRC : 37-2012

$$N = 4.1656 \times 10^{-0.08} \left[ \frac{1}{\epsilon_v} \right]^{4.5337}$$

$$N = 1.41 \times 10^{-8} \left[ \frac{1}{\epsilon_v} \right]^{4.5337}$$



where  $N$  = Number of cumulative standard axle  
 $\epsilon_v$  = Vertical strain in the subgrade



● ● ● End of Solution

**Q.22** A culvert is designed for a flood frequency of 100 years and a useful life of 20 years. The risk involved in the design of the culvert (in percentage, up to two decimal places) is \_\_\_\_\_.

**Ans. (18.20)**

Risk = The probability of a flood to occur at least once in  $n$ -successive years.

$$\begin{aligned} \therefore \text{Risk} &= 1 - q^n \\ &= 1 - (1 - P)^n \\ &= 1 - \left(1 - \frac{1}{T}\right)^n = 1 - \left(1 - \frac{1}{100}\right)^{20} \\ &= 1 - (0.99)^{20} \\ &= 0.18209 = 18.209\% \end{aligned}$$

● ● ● End of Solution

**Q.23** Dupuit's assumptions are valid for  
(a) artesian aquifer (b) confined aquifer  
(c) leaky aquifer (d) unconfined aquifer

**Ans. (d)**

Dupuit's theory assumptions hold that groundwater flows horizontally in an unconfined aquifer and that ground water discharge is proportional to saturated aquifer thickness.

● ● ● End of Solution

**Q.24** The quadratic equation  $2x^2 - 3x + 3 = 0$  is to be solved numerically starting with an initial guess as  $x_0 = 2$ . The new estimate of  $x$  after the first iteration using Newton-Raphson method is \_\_\_\_\_.

**Ans. (1)**

Given  $f(x) = 2x^2 - 3x + 3, x_0 = 2$   
 $f'(x) = 4x - 3$



By Newton-Raphson

$$x_1 = x_0 - \frac{f(x_0)}{f'(x_0)} = 2 - \frac{2(2)^2 - 3(2) + 3}{4(2) - 3}$$

$$= 2 - \frac{5}{5} = 1$$

• • • End of Solution

- Q.25** The setting time of cement is determined using
- (a) Le Chatelier apparatus (b) Briquette testing apparatus  
(c) Vicat apparatus (d) Casagrande's apparatus

**Ans. (c)**

Vicat apparatus is used to determine the normal consistency, IST, FST of cement.

• • • End of Solution

- Q.26** A coal containing 2% sulfur is burned completely to ash in a brick kiln at a rate of 30 kg/min. The sulfur content in the ash was found to be 6% of the initial amount of sulfur present in the coal fed to the brick kiln. The molecular weights of S, H and O are 32, 1 and 16 g/mole, respectively. The annual rate of sulfur dioxide (SO<sub>2</sub>) emission from the kiln (in tonnes/year, up to two decimal places) is \_\_\_\_\_.

**Ans. (592.88)**

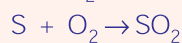
$$\text{Coal burned in one year} = 30 \times 24 \times 60 \times 365$$

$$= 1.5768 \times 10^7 \text{ kg}$$

$$\text{Sulfur content} = \frac{2}{100} \times 1.5768 \times 10^7 \times 10^{-6} = 315.36 \text{ tonnes/year}$$

$$\text{Sulfur content in ash} = \frac{6}{100} \times 315.36 = 18.92 \text{ tonnes/year}$$

$$\text{Sulfur converted to SO}_2 = 315.36 - 18.92 = 296.44 \text{ tonnes/year}$$



1 mole of S is present in 1 mole of SO<sub>2</sub>

32 gm of S is present in 64 gm of SO<sub>2</sub>

$$\therefore \text{Rate of SO}_2 \text{ emission} = \frac{64}{32} \times 296.44 = 592.88 \text{ tonnes /year}$$

• • • End of Solution

- Q.27** A flocculation tank contains 1800 m<sup>3</sup> of water, which is mixed using paddles at an average velocity gradient  $G$  of 100/s. The water temperature and the corresponding dynamic viscosity are 30°C and  $0.798 \times 10^{-3}$  Ns/m<sup>2</sup>, respectively. The theoretical power required to achieve the stated value of  $G$  (in kW, up to two decimal places) is \_\_\_\_\_.

Ans. (14.36)

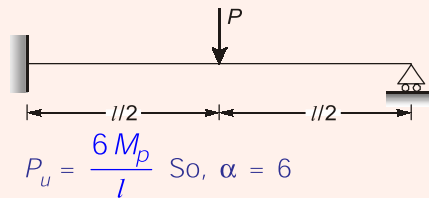
Power required

$$\begin{aligned} P &= \mu VG^2 \\ &= 0.798 \times 10^{-3} \text{ Ns/m}^2 \times 1800 \text{ m}^3 \times (1005)^2 \\ &= 14364 \text{ Nm/s or Watt} \\ &= 14.364 \text{ kW} \\ &= 14.36 \text{ kW} \end{aligned}$$

• • • End of Solution

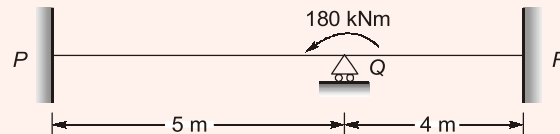
Q.28 A prismatic propped cantilever beam of span  $L$  and plastic moment capacity  $M$ , is subjected to a concentrated load at its mid-span. If the collapse load of the beam is  $\alpha \frac{M_p}{L}$ , the value of  $\alpha$  is \_\_\_\_\_.

Ans. (6)



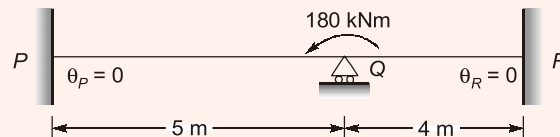
• • • End of Solution

Q.29 A prismatic beam P-Q-R of flexural rigidity  $EI = 1 \times 10^4 \text{ kNm}^2$  is subjected to a moment of 180 kNm at Q as shown in the figure.



The rotation at Q (in rad, up to two decimal places) is \_\_\_\_\_.

Ans. (0.01)



$$\begin{aligned} K_Q &= K_{QP} + K_{QR} \\ &= \frac{4EI}{5} + \frac{4EI}{4} = 1.8 EI \end{aligned}$$

$$\theta_Q = \frac{M_Q}{K_Q} = \frac{180}{1.8 \times 10^4} = 0.01 \text{ rad}$$



# MADE EASY

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# ESE-2018 Mains Batches

## Conventional Questions Practice Program

### +

## Mains Offline Test Series

Mains batches are exclusively designed for practice of conventional questions for ESE-2018. Although the syllabus of ESE pre & mains is well covered in classroom course, but still interested candidates can enroll in these batches to develop additional skills in order to excel in main examination. The approach followed in these batches are very beneficial to improve answer writing skills and special emphasis is given on presentation of answers. These batches are supplemented by well-designed ESE-2018 mains offline test series as per UPSC-QCAB pattern.

### Key Features

- Very useful to develop numerical solving approach & improving writing skills.
- Discussion on probable questions.
- Helps to develop step by step question solving approach.
- Comprehensive and in-depth discussion on collection of conventional questions, thus strengthening fundamental concepts.
- Special focus on improving answer layout specially for theory questions.
- Classes will be delivered by senior faculties.
- **Updated Mains workbook for every subject having varied practice question sets (unsolved and solved).**
- Test series will be conducted on every Sunday in synchronisation with the syllabus taught in classes.

### Batch Details

#### COURSE DURATION

70 to 80 days | 250-300 hours

#### CLASS DURATION

5-6 days a week and 6-7 hours a day

#### TEST SERIES

Every Sunday evening

Streams	Centre (Delhi)	Batch Type	Date	Timings
ME	Ghitorni Centre	Regular	<b>5<sup>th</sup> March, 2018</b>	7:30 AM to 1:30 PM
CE	Kalu Sarai	Regular	<b>Batches Commencing from 25<sup>th</sup> February, 2018</b>	7:30 AM to 1:30 PM
EE	Kalu Sarai (Choudhary House)	Regular		7:30 AM to 1:30 PM
E & T	Lado Sarai Centre	Regular		7:30 AM to 1:30 PM

Program	Commencing Date	Ex. MADE EASY Students <small>Enrolled in Postal, Rank Improvement, Conventional, GS, Post-GATE, GATE, I+G+P Batches</small>	Non MADE EASY students
Mains Exclusive Batch Inclusive of Mains Classroom Test Series for ESE-2018	<b>25th February, 2018</b>	₹ 12,500	₹ 16,500
<b>Mains Test Series</b> (Offline/Online)	<b>18th March, 2018</b>	₹ 2,000	₹ 3,000

## ADMISSION OPEN

44-A/1, Kalu Sarai, Sarvapriya Vihar,  
New Delhi - 110016

### Documents Required

- 2 Photographs + Valid photo ID proof
- Ex MADE EASY students should produce their MADE EASY ID card

011-45124612

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**Note : These batches will be conducted at Delhi centre only**

Alternate solution

$$M_{QP} = M_{IQP} + \frac{2EI}{l}[2\theta_Q + \theta_P] \quad \{M_{IQP} = 0, \theta_P = 0\}$$

$$= \frac{4 \times 10^4}{5} \times \theta_Q = 80000\theta_Q$$

$$M_{QR} = M_{IQR} + \frac{2EI}{l}[2\theta_Q + \theta_R] \quad \{M_{IQR} = 0, \theta_R = 0\}$$

$$= \frac{2 \times 10^4}{4}[2\theta_Q] = 10000\theta_Q$$

$$\Sigma M_Q = 0$$

$$\Rightarrow M_{QP} + M_{QR} + 180 = 0$$

$$\Rightarrow 18000 \cdot \theta_Q = -180$$

$$\Rightarrow \theta_Q = 0.01 \text{ radian (anticlockwise)}$$

• • • End of Solution

Q.30 The matrix  $\begin{pmatrix} 2 & -4 \\ 4 & -2 \end{pmatrix}$  has

- (a) real eigenvalues and eigenvectors
- (b) real eigenvalues but complex eigenvectors
- (c) complex eigenvalues but real eigenvectors
- (d) complex eigenvalues and eigenvectors

Ans. (d)

$$A = \begin{bmatrix} 2 & -4 \\ 4 & -2 \end{bmatrix}$$

$$|A - \lambda I| = 0$$

$$\begin{vmatrix} 2-\lambda & -4 \\ 4 & -2-\lambda \end{vmatrix} = 0$$

$$-4 - 2\lambda + 2\lambda + \lambda^2 + 16 = 0$$

$$\lambda^2 + 12 = 0$$

$$\lambda = \pm 2\sqrt{3}i$$

(Complex eigen values)

$$(1) \quad \lambda = 2\sqrt{3}i$$

$$\text{Consider } (A - \lambda I)X = 0$$

$$\begin{bmatrix} 2-2\sqrt{3}i & -4 \\ 4 & -2-2\sqrt{3}i \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

$$2 - 2\sqrt{3}i x_1 = 4x_2$$

$$\frac{x_1}{4} = \frac{x_2}{2 - 2\sqrt{3}i}$$

$$\begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 4 \\ 2 - 2\sqrt{3}i \end{bmatrix}$$

(2)  $\lambda = -2\sqrt{3}i$

Consider  $(A - \lambda I)X = 0$

$$\begin{bmatrix} 2 + 2\sqrt{3}i & -4 \\ 4 & -2 + 2\sqrt{3}i \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

$$2 + 2\sqrt{3}i x_1 = 4x_2$$

$$\frac{x_1}{4} = \frac{x_2}{2 + 2\sqrt{3}i}$$

$$\begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 4 \\ 2 + 2\sqrt{3}i \end{bmatrix}$$

$\therefore$  Complex Eigen values and complex Eigen vectors.

• • • End of Solution

**Q.31** The compression curve (void ratio,  $e$  vs. effective stress,  $\sigma'_v$ ) for a certain clayey soil is a straight line in a semi-logarithmic plot and it passes through the points ( $e = 1.2$ ;  $\sigma'_v = 50$  kPa) and ( $e = 0.6$ ;  $\sigma'_v = 800$  kPa). The compression index (up to two decimal places) of the soil is \_\_\_\_\_.

**Ans. (0.49)**

$$\text{Compression Index } C_c = \frac{\Delta e}{\log\left(\frac{\bar{\sigma}_1}{\bar{\sigma}_0}\right)} = \frac{1.2 - 0.6}{\log\left(\frac{800}{50}\right)}$$

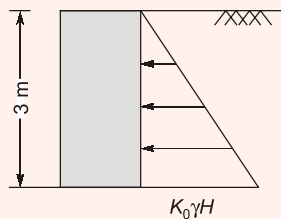
$$C_c = \frac{0.6}{\log(16)} = \frac{0.6}{1.204} = 0.4982$$

• • • End of Solution

**Q.32** A 3 m high vertical earth retaining wall retains a dry granular backfill with angle of internal friction of  $30^\circ$  and unit weight of  $20 \text{ kN/m}^3$ . If the wall is prevented from yielding (no movement), the total horizontal thrust (in kN per unit length) on the wall is

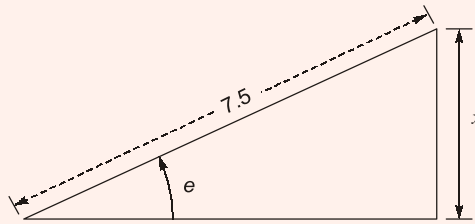
- (a) 0 (b) 30  
(c) 45 (d) 270

**Ans. (c)**





Ans. (0.525)



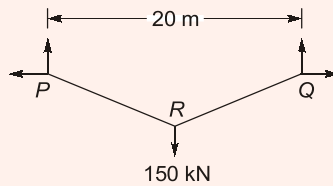
$$e = \frac{V^2}{225R} = \frac{100^2}{225 \times 510} = 0.0871 \approx 0.07 = \tan \theta$$

$$\sin \theta \approx 0.07$$

$$x = 7.5 \times \sin \theta = 0.525 \text{ m}$$

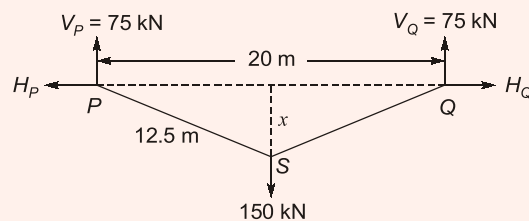
● ● ● End of Solution

**Q.35** A cable  $PQ$  of length 25 m is supported at two ends at the same level as shown in the figure. The horizontal distance between the supports is 20 m. A point load of 150 kN is applied at point  $R$  which divides it into two equal parts.



Neglecting the self-weight of the cable, the tension (in kN, integer value) in the cable due to the applied load will be \_\_\_\_\_.

Ans. (125)



$$12.5^2 = x^2 + 10^2 \Rightarrow x = 7.5 \text{ m}$$

Bending moment at  $S = 0$  {Consider the left part}

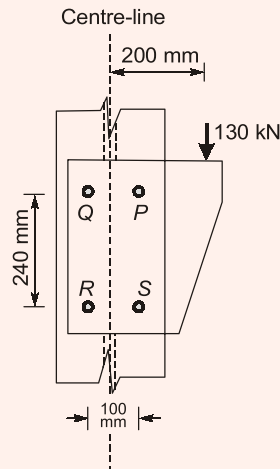
$$H_p \times 7.5 = 75 \times 10$$

$$H_p = 100 \text{ kN}$$

$$\text{Tension in cable} = \sqrt{H_p^2 + V_p^2} = \sqrt{75^2 + 100^2} = 125 \text{ kN}$$

● ● ● End of Solution

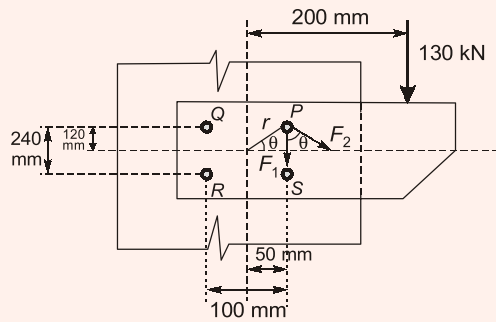
**Q.36** Four bolts P, Q, R and S of equal diameter are used for a bracket subjected to a load of 130 kN as shown in the figure.



The force in bolt P is

- (a) 32.50 kN (b) 69.32 kN  
(c) 82.50 kN (d) 119.32 kN

**Ans. (b)**



$$F_1 = \frac{P}{n} = \frac{130}{4} = 32.5 \text{ kN}$$

$$F_2 = \frac{Pe}{\sum r_i^2} \times r_p$$

$$r_p = r_Q = r_R = r_S = \sqrt{50^2 + 120^2} = 130 \text{ mm}$$

$$F_2 = \frac{(130 \times 200) \times 130}{4 \times 130^2} = \frac{200}{4} = 50 \text{ kN}$$

$$\cos \theta = \frac{50}{130}$$

$$\Rightarrow F_R = \sqrt{(32.5)^2 + (50)^2 + (2 \times 32.5 \times 50) \times \left(\frac{50}{130}\right)} = 69.33 \text{ kN}$$

• • • End of Solution



**Q.37** An aerial photograph of a terrain having an average elevation of 1400 m is taken at a scale of 1:7500. The focal length of the camera is 15 cm. The altitude of the flight above mean sea level (in m, up to one decimal place) is \_\_\_\_\_.

**Ans. (2525)**

$$h = 1400 \text{ m}$$

$$\text{Scale} = 1 : 7500$$

$$f = 15 \text{ cm}$$

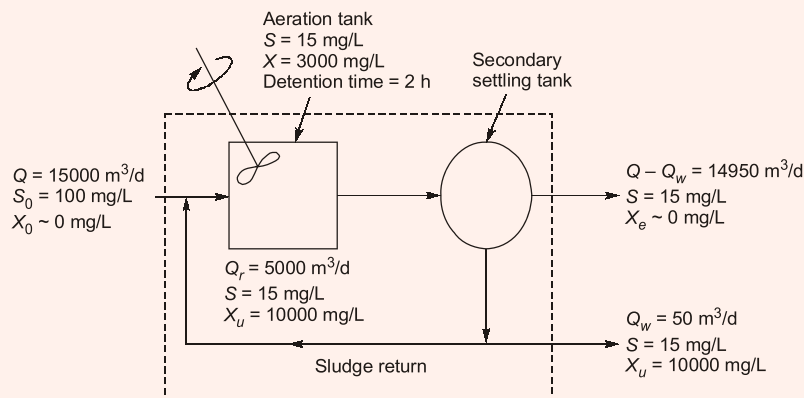
$$\text{Scale} = \frac{f}{H-h}$$

$$\Rightarrow \frac{1}{7500} = \frac{15 \times 10^{-2}}{H-1400}$$

$$\Rightarrow H = 2525 \text{ m}$$

• • • **End of Solution**

**Q.38** A schematic flow diagram of a completely mixed biological reactor with provision for recycling of solids is shown in the figure.



$S_0, S$  = readily biodegradable soluble BOD, mg/L

$Q, Q_r, Q_w$  = flow rates, m<sup>3</sup>/d

$X_0, X, X_e, X_u$  = microorganism concentrations (mixed-liquor volatile suspended solids or MLVSS), mg/L

The mean cell residence time (in days, up to one decimal place) is \_\_\_\_\_.

**Ans. (7.5)**

$$\theta_c = \frac{VX}{(Q_0 - Q_w)X_e + Q_w X_u}$$

$$= \frac{Q_0 HRT X}{(Q_0 - Q_w)X_e + Q_w X_u} \quad (X_e = 0)$$

$$= \frac{15000 \times \frac{2}{24} \times 3000}{50 \times 10000} = 7.5 \text{ days}$$

• • • **End of Solution**

- Q.39 The rank of the following matrix is  $\begin{pmatrix} 1 & 1 & 0 & -2 \\ 2 & 0 & 2 & 2 \\ 4 & 1 & 3 & 1 \end{pmatrix}$
- (a) 1 (b) 2  
(c) 3 (d) 4

Ans. (b)

$$A = \begin{pmatrix} 1 & 1 & 0 & -2 \\ 2 & 0 & 2 & 2 \\ 4 & 1 & 3 & 1 \end{pmatrix}$$

$$R_2 \rightarrow R_2 - 2R_1, R_3 \rightarrow R_3 - 4R_1$$

$$\begin{pmatrix} 1 & 1 & 0 & -2 \\ 0 & -2 & 2 & 6 \\ 0 & -3 & 3 & 9 \end{pmatrix}$$

$$R_3 \rightarrow R_3 - \frac{3}{2}R_2$$

$$\begin{pmatrix} 1 & 1 & 0 & -2 \\ 0 & -2 & 2 & 6 \\ 0 & 0 & 0 & 0 \end{pmatrix}$$

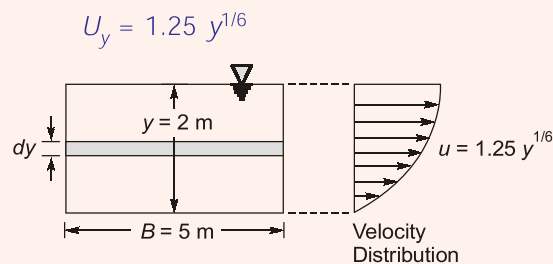
No. of non zero rows = 2

rank of  $A = 2$

• • • End of Solution

- Q.40 In a 5 m wide rectangular channel, the velocity  $u$  distribution in the vertical direction  $y$  is given by  $u = 1.25y^{1/6}$ . The distance  $y$  is measured from the channel bed. If the flow depth is 2 m, the discharge per unit width of the channel is
- (a) 2.40 m<sup>3</sup>/s/m (b) 2.80 m<sup>3</sup>/s/m  
(c) 3.27 m<sup>3</sup>/s/m (d) 12.02 m<sup>3</sup>/s/m

Ans. (a)



Discharge through elementary strip ( $dQ$ ) =  $5 dy \times u$

$$\therefore \text{Total discharge, } Q = \int_0^2 (5dy)(1.25y^{1/6})$$

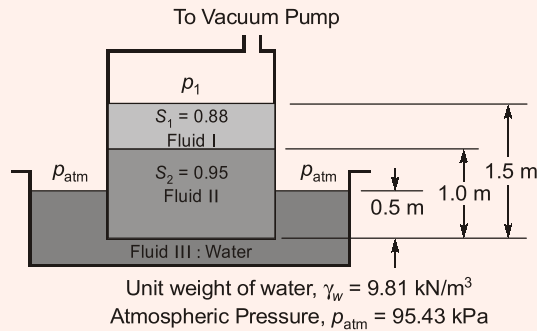
$$\Rightarrow Q = 6.25 \int_0^2 y^{1/6} dy = 6.25 \times \left[ \frac{y^{1/6+1}}{\frac{1}{6}+1} \right]_0^2$$

$$\therefore Q = 12.026 \text{ m}^3$$

$$\therefore \text{Discharge per unit width, } q = \frac{12.026}{5} = 2.405 \text{ m}^3/\text{s/m}$$

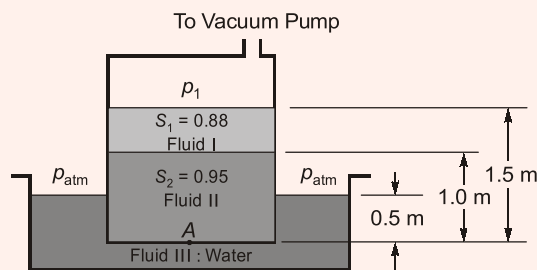
• • • End of Solution

**Q.41** A three-fluid system (immiscible) is connected to a vacuum pump. The specific gravity values of the fluids ( $S_1$ ,  $S_2$ ) are given in the figure.



The gauge pressure value (in  $\text{kNm}^2$ , up to two decimal places) of  $p_1$  is \_\_\_\_\_.

**Ans. (-8.73)**



Taking  $P_1$  is in gauge pressure.

$$P_A = P_1 + (0.88 \times 10^3) \cdot (9.81)(0.5) + (0.95 \times 10^3)(9.81)(1)$$

$$(10^3)(9.81)(0.5) = P_1 + (0.88 \times 10^3) \cdot (9.81)(0.5) + (0.95 \times 10^3)(9.81)(1)$$

$$P_1 = -8.73 \text{ kN/m}^2$$

• • • End of Solution

**Q.42** A rough pipe of 0.5 m diameter, 300 m length and roughness height of 0.25 mm, carries water (kinematic viscosity =  $0.9 \times 10^{-6} \text{ m}^2/\text{s}$ ) with velocity of 3 m/s. Friction factor ( $f$ ) for laminar flow is given by  $f = 64/Re$ , and for turbulent flow it is given by

$$\frac{1}{\sqrt{f}} = 2 \log_{10} \left( \frac{r}{k} \right) + 1.74, \text{ where, } Re = \text{Reynolds number, } r = \text{radius of pipe, } k = \text{roughness}$$

height and  $g = 9.81 \text{ m/s}^2$ . The head loss (in m, up to three decimal places) in the pipe due to friction is \_\_\_\_\_.

**Ans. (4.594)**

$$Re = \frac{\rho \cdot V \cdot D}{\mu} = \frac{V \cdot D}{\nu} = \frac{3 \times (0.5)}{0.9 \times 10^{-6}} = 1.67 \times 10^6$$

Means  $Re > 2000$  turbulent flow.

$$\text{So, } \frac{1}{\sqrt{f}} = 2 \log_{10} \frac{D}{2k_s} + 1.74$$

$$\frac{1}{\sqrt{f}} = 2 \log_{10} \frac{0.5}{2 \times 0.25 \times 10^{-3}} + 1.74$$

$$f = 0.01669$$

$$h_f = \frac{f \cdot L \cdot V^2}{2gD} = \frac{(0.01669)(300)(3)^2}{2 \times 9.81 \times 0.5} = 4.594 \text{ m}$$

• • • **End of Solution**

**Q.43** The total horizontal and vertical stresses at a point  $X$  in a saturated sandy medium are 170 kPa and 300 kPa, respectively. The static pore-water pressure is 30 kPa. At failure, the excess pore-water pressure is measured to be 94.50 kPa. and the shear stresses on the vertical and horizontal planes passing through the point  $X$  are zero. Effective cohesion is 0 kPa and effective angle of internal friction is  $36^\circ$ . The shear strength (in kPa, up to two decimal places) at point  $X$  is \_\_\_\_\_.

**Ans. (52.52)**

$$\therefore \sigma_n = \frac{300+170}{2} + \frac{300-170}{2} \cos 2\alpha$$

$$\alpha = 45^\circ + \frac{36^\circ}{2} = 63^\circ$$

$$\Rightarrow \sigma_n = 196.79 \text{ kPa}$$

$$\sigma'_n = \sigma_n - u = 196.79 - (30 + 94.5)$$

$$\sigma'_n = 72.29 \text{ kPa}$$

$$\Rightarrow \tau = C + \sigma'_n \tan \phi = 72.29 \tan 36 = 52.52 \text{ kPa}$$

• • • **End of Solution**

**Q.44** The value (up to two decimal places) of a line integral  $\int_C \vec{F}(\vec{r}) \cdot d\vec{r}$ , for  $\vec{F}(\vec{r}) = x^2\vec{i} + y^2\vec{j}$  along  $C$  which is a straight line joining (0, 0) to (1, 1) is \_\_\_\_\_.

**Ans.** (0.666)

$$\vec{F} = x^2\vec{i} + y^2\vec{j}$$

$$\int \vec{F} \cdot d\vec{r} = \int (x^2\vec{i} + y^2\vec{j}) \cdot (dx\vec{i} + dy\vec{j})$$

$$= \int x^2 dx + y^2 dy$$

(0, 0) to (1, 1) line is  $y = x$

$$= \int x^2 dx + x^2 dx = \int_0^1 2x^2 dx$$

$$= 2 \left( \frac{x^3}{3} \right) \Big|_0^1 = \frac{2}{3} = 0.666$$

● ● ● End of Solution

**Q.45** A car follows a slow moving truck (travelling at a speed of 10 m/s) on a two-lane two-way highway. The car reduces its speed to 10 m/s and follows the truck maintaining a distance of 16 m from the truck. On finding a clear gap in the opposing traffic stream, the car accelerates at an average rate of 4 m/s<sup>2</sup>, overtakes the truck and returns to its original lane. When it returns to its original lane, the distance between the car and the truck is 16 m. The total distance covered by the car during this period (from the time it leaves its lane and subsequently returns to its lane after overtaking) is

- (a) 64 m (b) 72 m  
(c) 128 m (d) 144 m

**Ans.** (b)

Overtaking time,  $T = \sqrt{\frac{4s}{a}} = \sqrt{\frac{4 \times 16}{4}} = 4 \text{ sec}$

$S =$  Space headway = 16 m

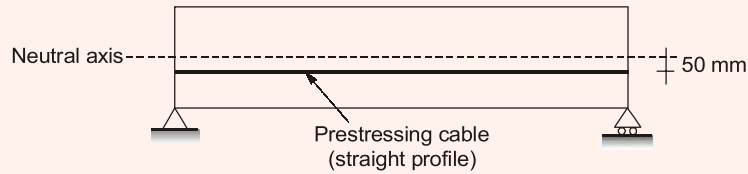
$a =$  Acceleration = 4 m/s<sup>2</sup>

Distance travelled by vehicle =  $S_2$

$$S_2 = uT + \frac{1}{2}aT^2 = 10 \times 4 + \frac{1}{2} \times 4 \times 4^2 = 72 \text{ m}$$

● ● ● End of Solution

**Q.46** A 6 m long simply-supported beam is prestressed as shown in the figure.



The beam carries a uniformly distributed load of 6 kN/m over its entire span. If the effective flexural rigidity  $EI = 2 \times 10^4$  kNm<sup>2</sup> and the effective prestressing force is 200 kN, the net increase in length of the prestressing cable (in mm, up to two decimal places) is \_\_\_\_\_.

**Ans. (0.12)**

Span of PSC beam = 6 m

$$EI = 2 \times 10^4 \text{ kNm}^2 = 2 \times 10^4 \times 1000^3 \text{ N-mm}^2 \\ = 2 \times 10^{13} \text{ N-mm}^2$$

$$P = 200 \text{ kN}$$

Total UDL = 6 kN/m

eccentricity =  $e = 50$  mm

(a) Slope of beam due to P-force

$$\theta_1 = \frac{P \cdot e \cdot L}{2EI} \\ = \frac{200 \times 10^3 \times 50 \times 6000}{2 \times 2 \times 10^{13}} \\ = 1.5 \times 10^{-3} \text{ (upward)}$$

(b) Slope of beam due to UDL

$$\theta_2 = \frac{wL^3}{24EI} \\ = \frac{6 \times (6000)^3}{24 \times 2 \times 10^{13}} = (+) 2.7 \times 10^{-3} \text{ (downward)}$$

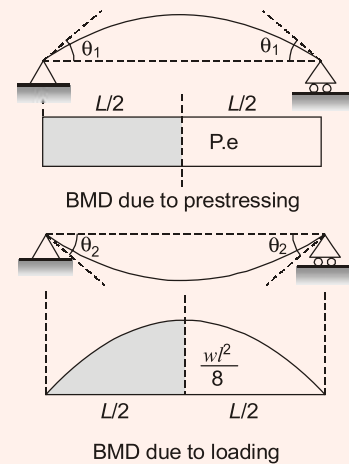
(c) Net slope of beam

$$\theta = \theta_1 + \theta_2 \\ = (-) 1.5 \times 10^{-3} + (+) 2.7 \times 10^{-3} \\ = 1.2 \times 10^{-3}$$

(d)



Total net increase in length

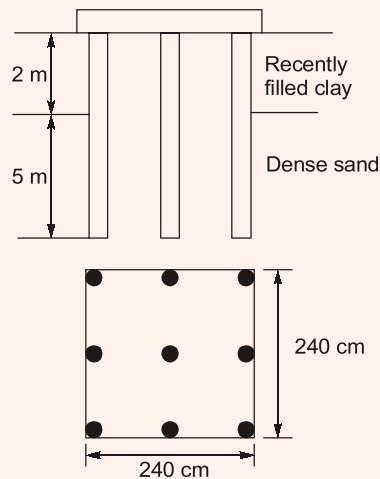


$$= 2e\theta$$

$$= 2 \times 50 \times 1.2 \times 10^{-3} = 0.12 \text{ mm}$$

● ● ● End of Solution

**Q.47** A group of nine piles in a 3 × 3 square pattern is embedded in a soil strata comprising dense sand underlying recently filled clay layer, as shown in the figure. The perimeter of an individual pile is 126 cm. The size of pile group is 240 cm × 240 cm. The recently filled clay has undrained shear strength of 15 kPa and unit weight of 16 kN/m<sup>3</sup>.



The negative frictional load (in kN, up to two decimal places) acting on the pile group is \_\_\_\_\_.

**Ans. (472.32)**

3 × 3 pile group

Negative skin friction for group.

$$= \alpha \bar{c}(4BL) + \text{weight of soil in negative zone.}$$

$$= \alpha \bar{c}(4BL) + \gamma[\text{Area} \times \text{length}]$$

$$= 1 \times 15[4 \times 2.4 \times 2] + 16[2.4^2 \times 2] = 472.32 \text{ kN/m}^2$$

Negative skin friction in individual action.

$$= n[\alpha \bar{c} (\text{perimeter} \times l)]$$

$$= 9[0.5 \times 15 \times 1.26 \times 2] \quad [\text{Assume } \alpha = 0.5]$$

$$= 170.1 \text{ kN/m}^2$$

Negative skin friction is maximum of above (two)

$$Q_{nf} = 472.32 \text{ kN/m}^2$$

● ● ● End of Solution

**Q.48** The Laplace transform  $F(s)$  of the exponential function.  $f(t) = e^{at}$  when  $t \geq 0$ , where  $a$  is a constant and  $(s - a) > 0$ , is

- (a)  $\frac{1}{s+a}$  (b)  $\frac{1}{s-a}$   
(c)  $\frac{1}{a-s}$  (d)  $\infty$

**Ans.** (b)

$$L(e^{at}) = \frac{1}{s-a}$$

$$L(a^{at}) = \int_0^{\infty} e^{-st} e^{at} dt = \int_0^{\infty} e^{-(s-a)t} dt$$

$$= \left. \frac{e^{-(s-a)t}}{-(s-a)} \right|_0^{\infty} = -\frac{1}{s-a}(0-1) = \frac{1}{s-a}$$

● ● ● **End of Solution**

**Q.49** At a small water treatment plant which has 4 Filters, the rates of filtration and backwashing are  $200 \text{ m}^3/\text{d}/\text{m}^2$  and  $1000 \text{ m}^3/\text{d}/\text{m}^2$ , respectively. Backwashing is done for 15 min per day. The maturation, which occurs initially as the filter is put back into service after cleaning, takes 30 min. It is proposed to recover the water being wasted during backwashing and maturation. The percentage increase in the filtered water produced (up to two decimal places) would be \_\_\_\_\_.

**Ans.** (7.525)

Let total area of filters be  $1 \text{ m}^2$

$$\text{Water used for backwashing} = 1000 \times \frac{15}{24 \times 60} = 10.4166 \text{ m}^3$$

$$\text{Water used for maturation} = 200 \times \frac{30}{24 \times 60} = 4.166 \text{ m}^3$$

$$\begin{aligned} \text{Total water wasted for backwashing and maturation} \\ = 10.4166 + 4.166 = 14.58 \text{ m}^3 \end{aligned}$$

$$\text{Water to be treated by filtered} = 200 \times \frac{23.25}{24} = 193.75 \text{ m}^3/\text{day}$$

$$\% \text{ increase in filtered water produced} = \frac{14.58}{193.75} \times 100 = 7.525\%$$

● ● ● **End of Solution**

**Q.50** The space mean speed (kmph) and density (vehicles/km) of a traffic stream are linearly related. The free flow speed and jam density are 80 kmph and 100 vehicles/km respectively. The traffic flow (in vehicles/h, up to one decimal place) corresponding to a speed of 40 kmph is \_\_\_\_\_.



Ans. (2000)

$$V_f = \text{Free mean speed} = 50 \text{ kmph}$$

$$k_j = \text{Jam density} = 100 \text{ veh/km}$$

As per linear model (green-shield)

$$V = V_f \left(1 - \frac{k}{k_j}\right)$$

⇒ Density @ speed = 40 kmph

$$40 = 50 \left(1 - \frac{k}{100}\right) = 50 \text{ veh/km}$$

Traffic flow @ density 50 veh/km @ speed 40 kmph

$$q = Vk$$

$$q = V_f \left(k - \frac{k^2}{k_j}\right) \Rightarrow 80 \left(50 - \frac{50^2}{100}\right) = 2000 \text{ veh/hr}$$

• • • End of Solution

**Q.51** The total rainfall in a catchment of area 1000 km<sup>2</sup>, during a 6 h storm, is 19 cm. The surface runoff due to this storm computed from triangular direct runoff hydrograph is  $1 \times 10^8 \text{ m}^3$ . The  $\phi_{\text{index}}$  for this storm (in cm/h, up to one decimal place) is \_\_\_\_\_.

Ans. (1.5)

$$\text{Surface runoff} = \frac{1 \times 10^8 \text{ m}^3}{1000 \times 10^6 \text{ m}^2} = 0.1 \text{ m} = 10 \text{ cm}$$

$$\text{Total Rainfall} = 19 \text{ cm}$$

$$\text{Rainfall intensity} = \frac{19}{6} = 3.167 \text{ cm/hr}$$

$$w\text{-index} = \frac{P - Q}{t} = \frac{\text{Total infiltration}}{\text{Total duration of storm}}$$

$$\therefore w\text{-index} = \frac{19 - 10}{6} = 1.5 \text{ cm/hr}$$

As intensity of rainfall > w-index.

And rainfall intensity is uniform therefore  $\phi\text{-index} = w\text{-index} = 1.5 \text{ cm/hr}$ .

• • • End of Solution

**Q.52** A level instrument at a height of 1.320 m has been placed at a station having a Reduced Level (RL) of 112.565 m. The instrument reads -2.835 m on a levelling staff held at the bottom of a bridge deck. The RL (in m) of the bottom of the bridge deck is

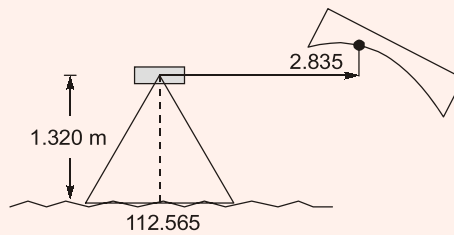
(a) 116.720

(b) 116.080

(c) 114.080

(d) 111.050

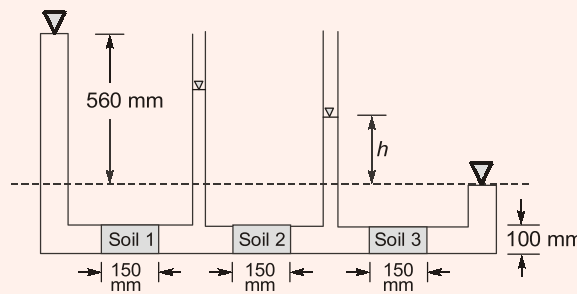
Ans. (a)



RL of bottom of bridge deck  
 $= 112.565 + 1.320 + (2.835)$   
 $= 116.720 \text{ m}$

● ● ● End of Solution

**Q.53** Three soil specimens (Soil 1, Soil 2 and Soil 3), each 150 mm long and 100 mm diameter, are placed in series in a constant head flow set-up as shown in the figure. Suitable screens are provided at the boundaries of the specimens to keep them intact. The values of coefficient of permeability of Soil 1, Soil 2 and Soil 3 are 0.01, 0.003 and 0.03 cm/s, respectively.



The value of  $h$  in the set-up is

- (a) 0 mm (b) 40 mm  
 (c) 255 mm (d) 560 mm

Ans. (b)

In normal to bedding plane flow (series arrangement), Discharge will be same and Head loss and Hydraulic gradient will be different.

$$q = K_1 i_1 A = K_2 i_2 A = K_3 i_3 A = K_{avg} \cdot \left(\frac{H_L}{L}\right) A$$

$$K_{avg} = \frac{\sum Z_i}{\sum \frac{Z_i}{K_i}} = \frac{150 + 150 + 150}{\frac{150}{0.01} + \frac{150}{0.003} + \frac{150}{0.03}} = 0.0064$$

Total head loss =  $H_L = 560 \text{ mm}$

$$\therefore K_3 \cdot \frac{h}{150} \times A = K_{avg} \cdot \frac{560}{(150 + 150 + 150)} A$$

$$0.03 \cdot \left(\frac{h}{150}\right) = 0.0064 \left(\frac{560}{450}\right)$$
$$h = 40 \text{ mm}$$

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**End of Solution**

**Q.54** A singly-reinforced rectangular concrete beam of width 300 mm and effective depth 400 mm is to be designed using M25 grade concrete and Fe500 grade reinforcing steel. For the beam to be under-reinforced, the maximum number of 16 mm diameter reinforcing bars that can be provided is

- (a) 3 (b) 4  
(c) 5 (d) 6

**Ans. (c)**

$B = 300 \text{ mm}$   
 $d = 400 \text{ mm}$  (effective depth)  
M25 and Fe500

$$A_{st, \text{lim}} = 0.414 \left(\frac{f_{ck}}{f_y}\right) x_{u, \text{lim}} b$$
$$= 0.414 \left(\frac{25}{500}\right) 0.46 \times 400 \times 300$$
$$= 1142.64 \text{ mm}^2$$

$$\text{Number of } 16 \text{ mm } \phi = \frac{1142.64}{\frac{\pi}{4}(16)^2} = 5.68$$

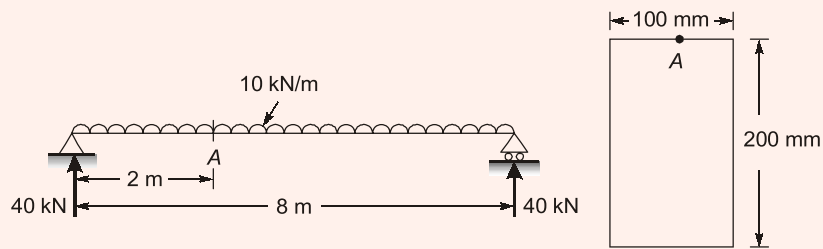
For  $A_{st} < A_{st, \text{lim}}$ , maximum number of bars to be provided is 5.

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**End of Solution**

**Q.55** An 8 m long simply-supported elastic beam of rectangular cross-section (100 mm × 200 mm) is subjected to a uniformly distributed load of 10 kN/m over its entire span. The maximum principal stress (in MPa, up to two decimal places) at a point located at the extreme compression edge of a cross-section and at 2 m from the support is \_\_\_\_\_.

Ans. (90)

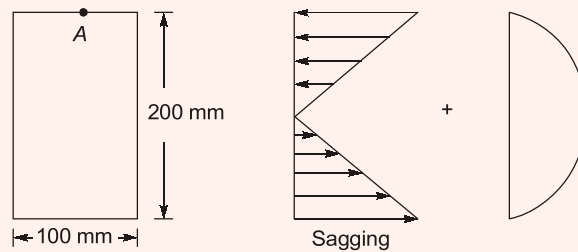


$$V_A + V_B = 80$$

$$V_A = V_B = 40$$

$$M_A = (-10 \times 2 \times 1) + 40 \times 2 = 60 \text{ kNm}$$

[Due to symmetry]



At extreme compression edge,

$$\text{Bending stress, } \sigma = \frac{My}{I} = \frac{(60 \times 10^6) \times \frac{200}{2}}{100 \times \frac{200^3}{12}} = 90 \text{ MPa}$$

Direct shear stress = 0

Principal stress,

$$\sigma_{p1} / \sigma_{p2} = \frac{90 + 0}{2} \pm \frac{1}{2} \sqrt{(0 - 90)^2 + y(0)^2}$$

$$\sigma_{p1} = 90 \text{ MPa}$$

So principal stress = 90 N/mm<sup>2</sup> = 90 MPa

● ● ● End of Solution

